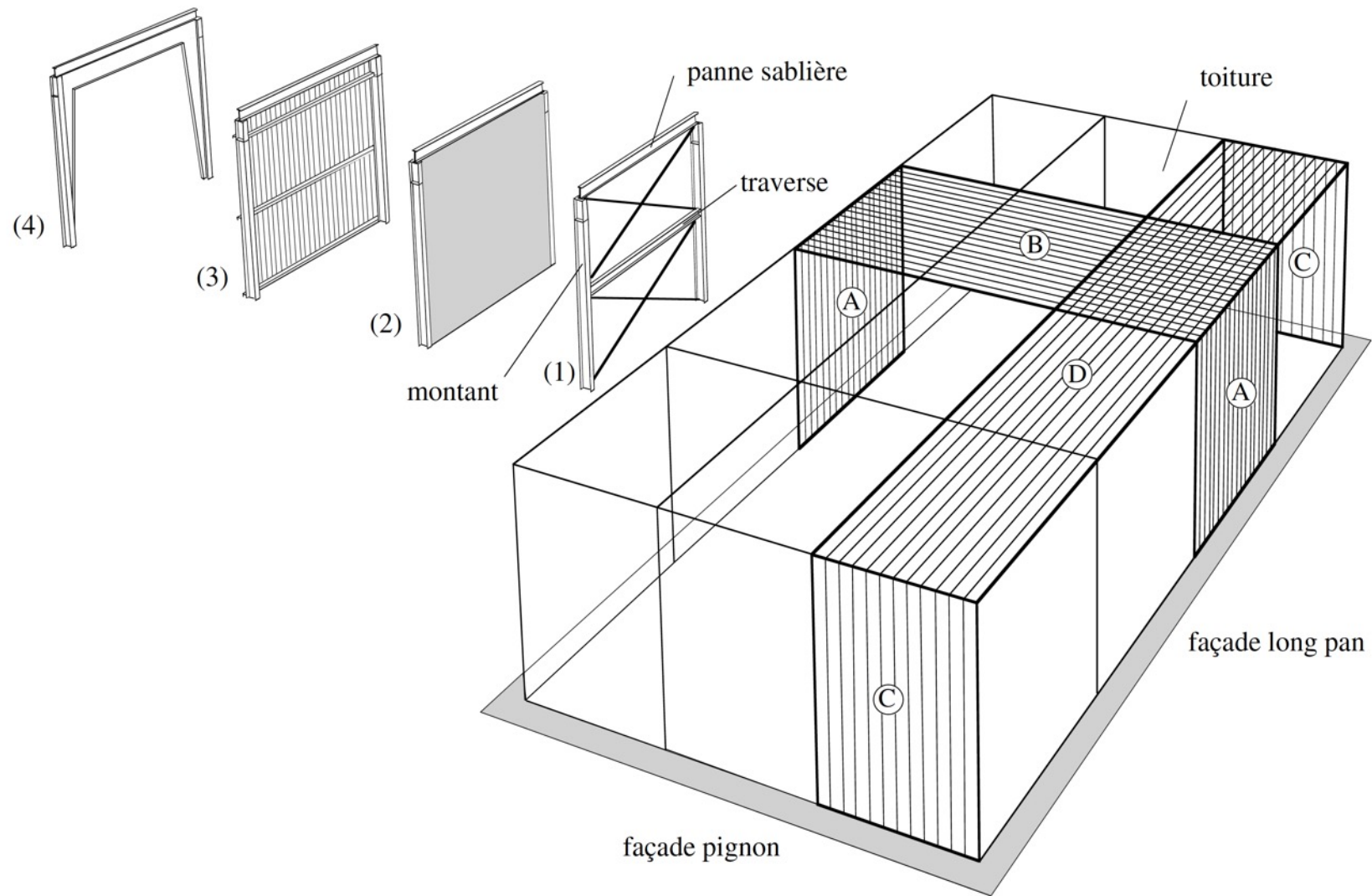


CIVIL-526: Steel structures, selected chapters

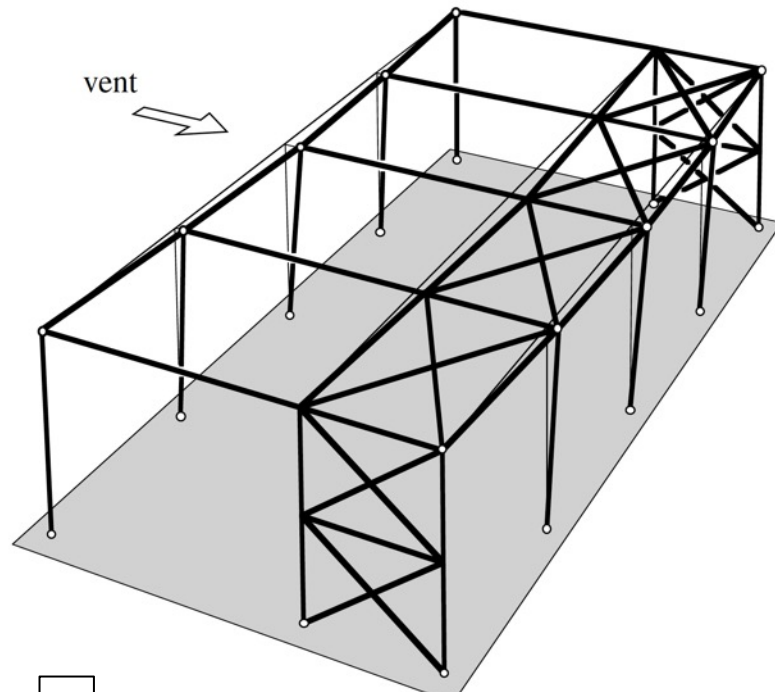
Design of load-bearing systems for halls and
buildings

- Design of indust. halls and buildings TGC 11, chap. 1-5
 - Stabilisation, design of load-bearing systems, Chaps. 3&4
beams
 - Load path to foundations Chaps. 6&13
- Building components in composite steel and concrete
 - composite beams, composite slabs, shear Chap. 10
connection (total, partial)
 - Joints: beam-column, frame joints, column Chap. 12
bases
 - Composite columns Chap. 13

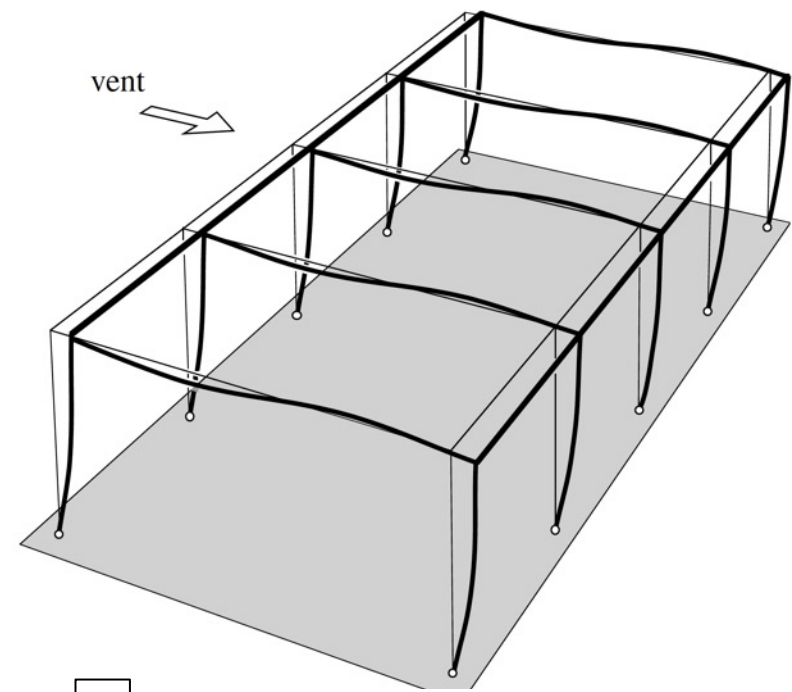
Reminder: bracing panels/walls (TGC 11, fig. 3.18)



Reminder: stabilisation of hinged VS rigid frames (Fig 3.22)



a Frames with 4 hinges
(bracings for stability)



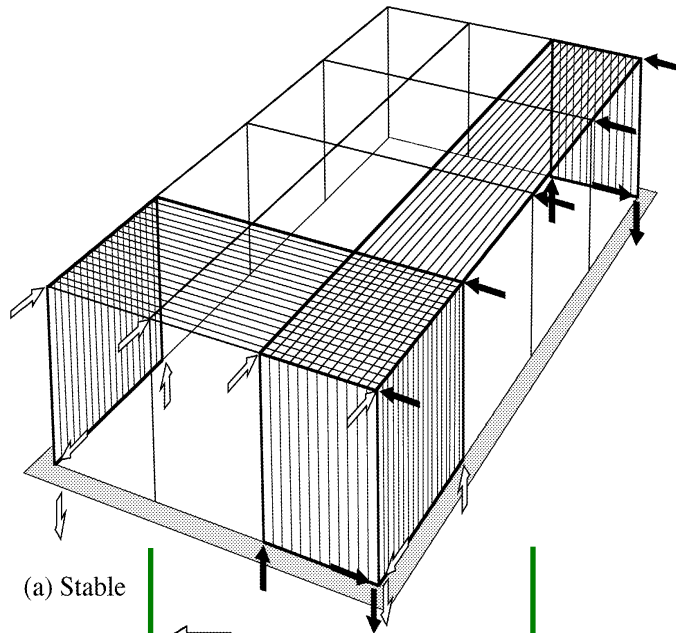
b Rigid frames
(stability thru frame effect)

- 1) Under wind loads, which statements are correct when comparing these 2 systems?
- A. System **a** is more rigid than system **b**
 - B. System **b** is lighter and more economical
 - C. In the case of a very long hall, system **b** is advantageous
 - D. The system **a** makes it easier to extend the length of the hall

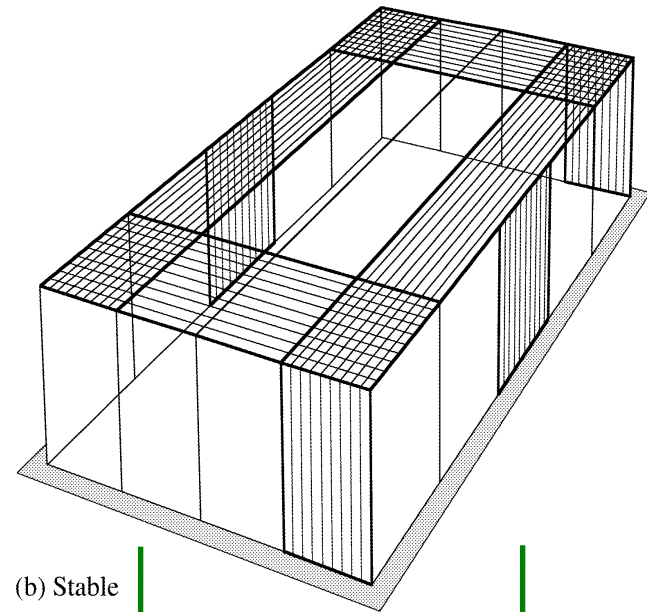
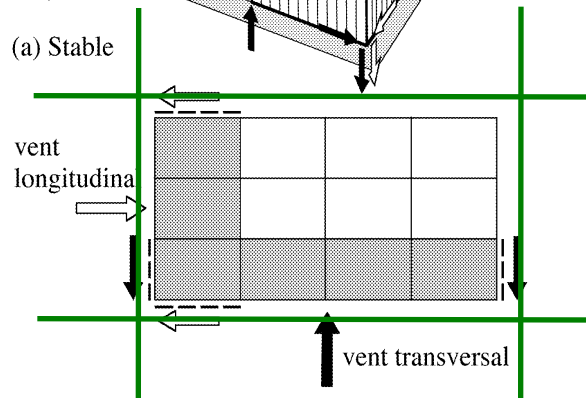
Reminder: bracing layout (TGC 11, fig. 3.19)

To balance a force acting in any direction :

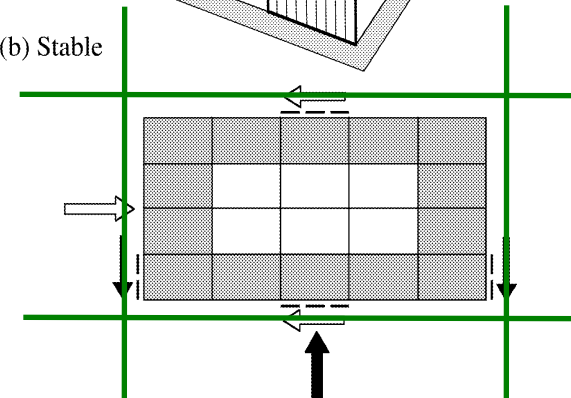
1. you need at least 3 lines of force,
2. the lines of action of the forces must not intersect at a point,
3. the lines of action of the forces must not be all parallel.



(a) Stable



(b) Stable



Reminder: bracing layout (TGC 11, fig. 3.19)

Bracing has 3 functions:

1. Transmit horizontal forces,
 2. Limit distortion,
 3. Help stabilise elements (beams to prevent tipping, LTB, columns to prevent buckling)
- } Global stability

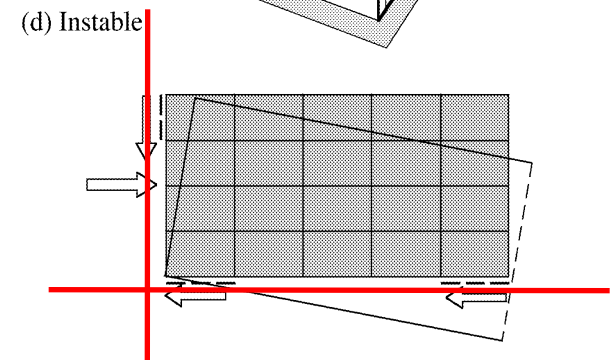
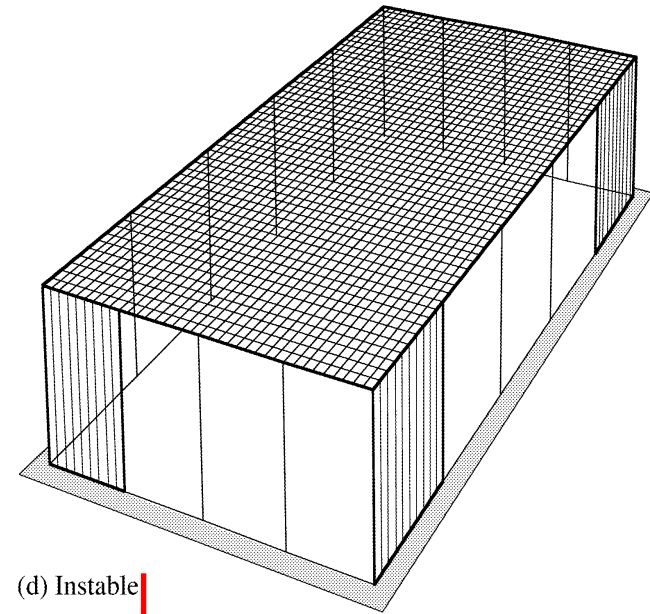
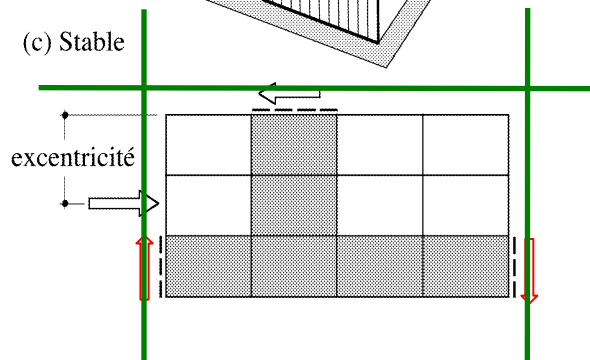
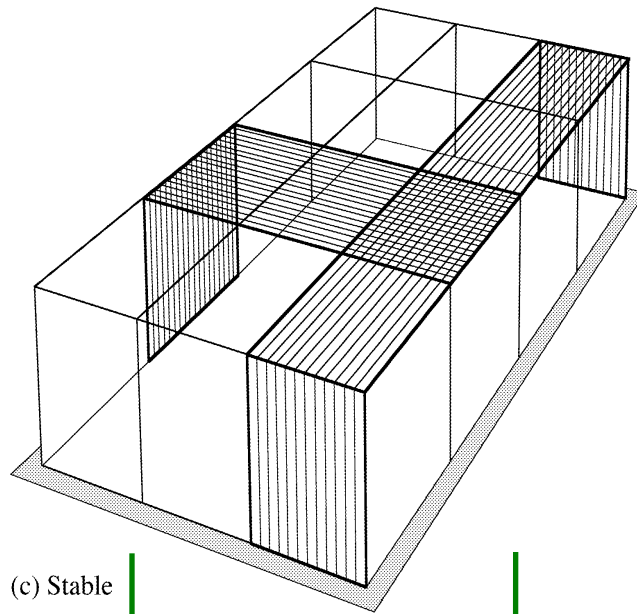
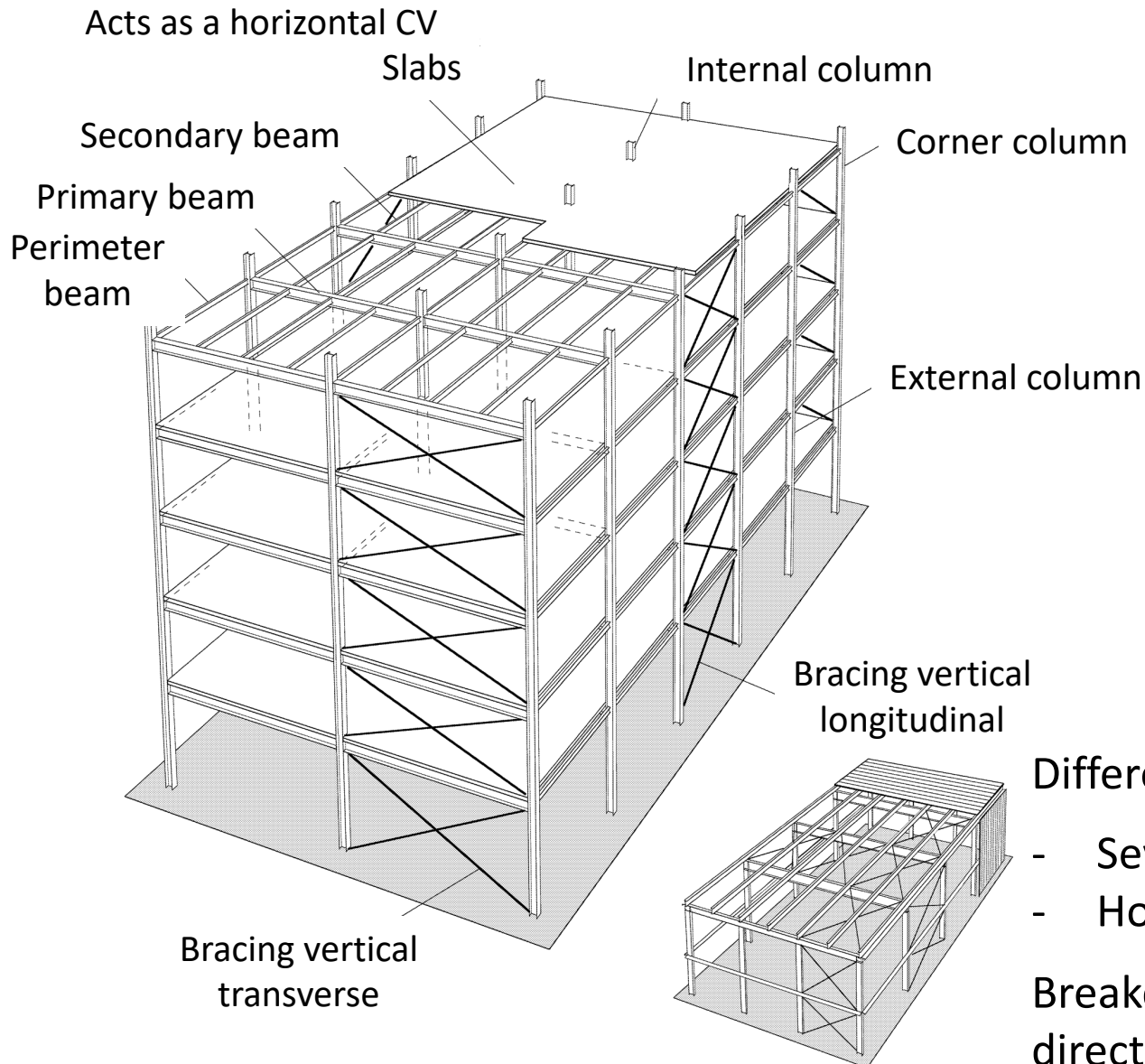


Fig. 4.1: Frame of multi-storey building



Vocabulary:

Primary beam – Sommier

Secondary beam – Solive

Purlin – Panne

Lattice girder – Poutre treillis

Slab – Dalle

Bracing – Contreventement
(CV)

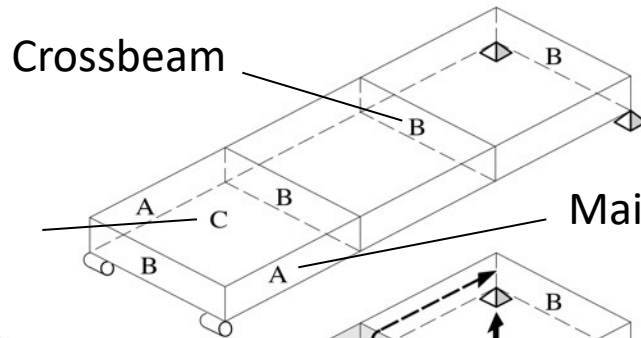
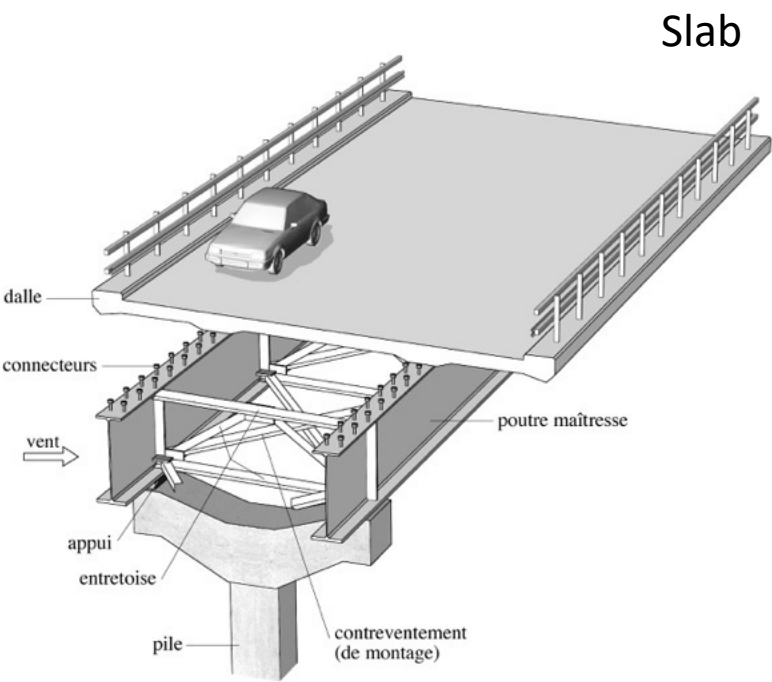
Differences with indust. buildings

- Several transverse bays
- Horizontal CV = Slabs

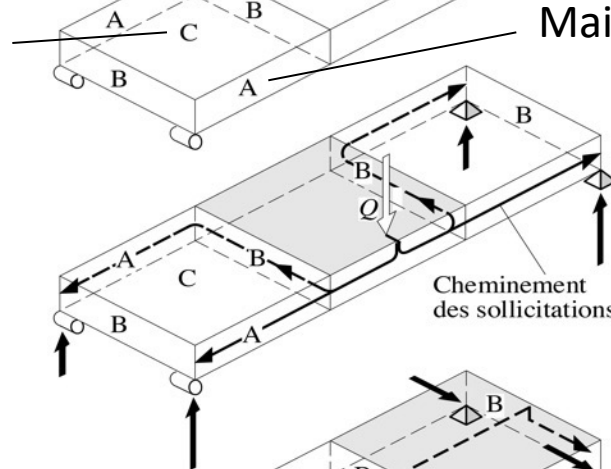
Breakdown into planes in 3 directions

Bridge: breakdown of structure into planes, loads paths

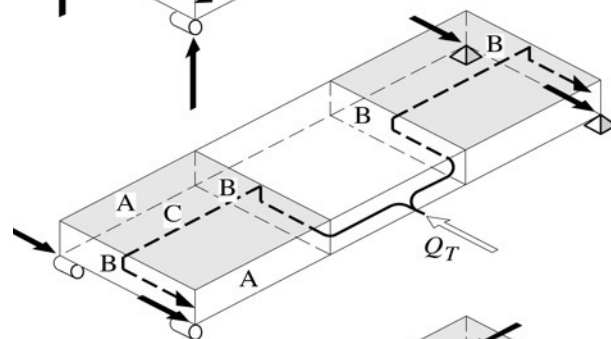
Example composite bridge: (TGC 12 Fig. 5.3)



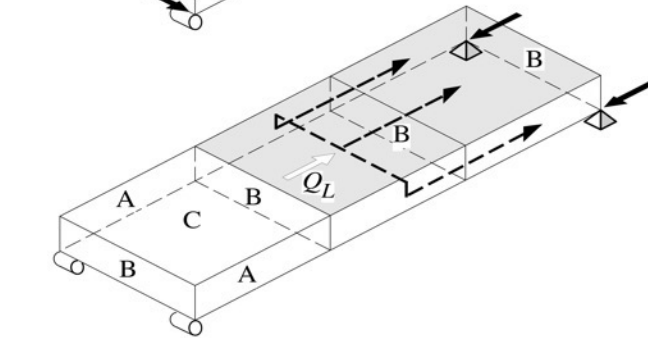
(a) Structure composed of planes



(b) Vertical load path



(c) Path of a horizontal transverse load



(d) Path of a horizontal longitudinal load

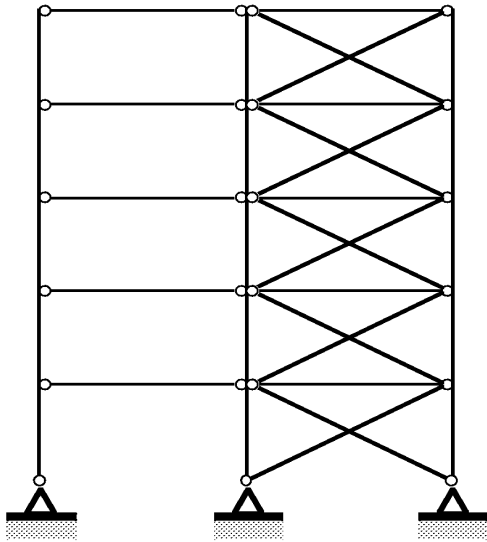
Vocabulary:
Main beam – Poutre maîtresse
Crossbeam – Entretoise
Bearing – Appui
Pier – Pile

The most common solution:

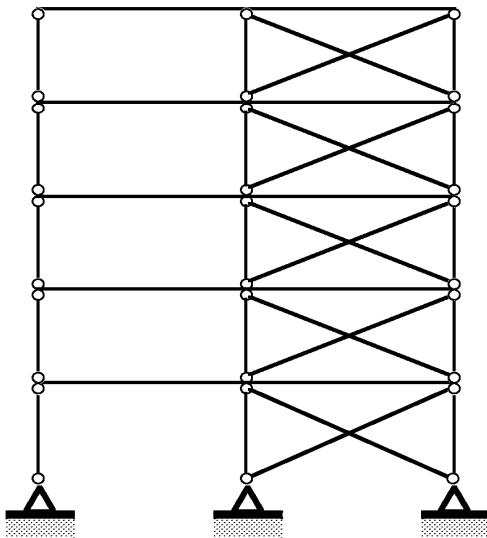
- two systems to resist the different load types applied to the structural system
- a semi-rigid or articulated framing system, which only resists vertical actions
- steel bracing or concrete walls/cores to resist horizontal actions.

- **Articulated frames** figs. 4.1 - 4.2
- **Frames with central core** figs. 4.3 - 4.4
- **Structures with rigid storeys (stiffening belt or roof with suspended floors)** figs. 4.5 - 4.7
- **Rigid frame structures** figs. 4.8 - 4.10
- Cost summaries, pre-sizing
- **Tall buildings. High-rise buildings, outer tube**
figs. 4.12 - 4.13

Fig. 4.2: Articulated (and braced) frames



(b) Poteaux continus



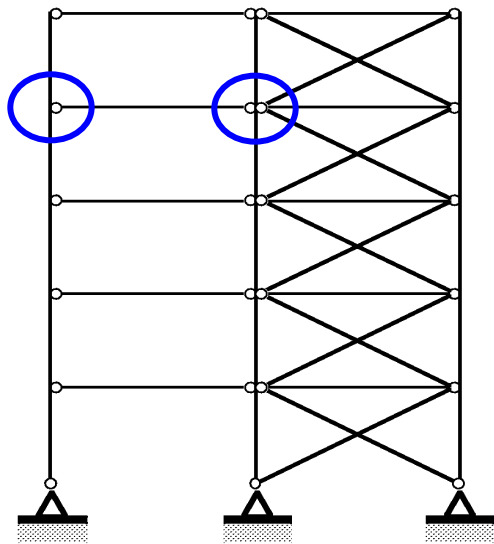
(c) Sommier continu

- ✓ Simple design and execution
- ✓ Quick assembly (to be stabilised at each floor)
- ✓ System not very sensitive to tolerances
- ✓ Continuous columns. N

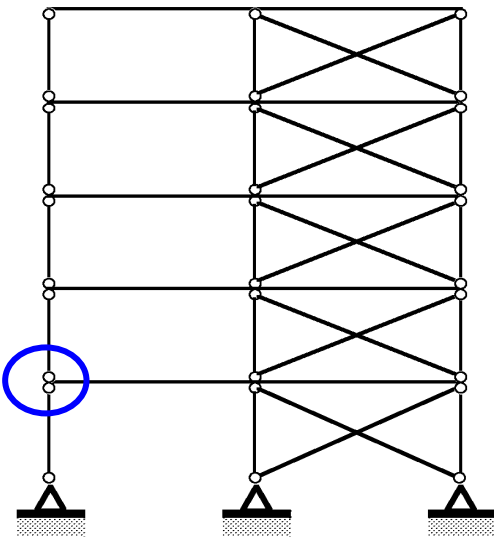
- x High and heavy beams
- x In case of interrupted columns, what about transfer of significant vertical forces?
- x Not very robust
- x CV strong presence

Joints ? Figs. 4.41, 4.43 and 4.44

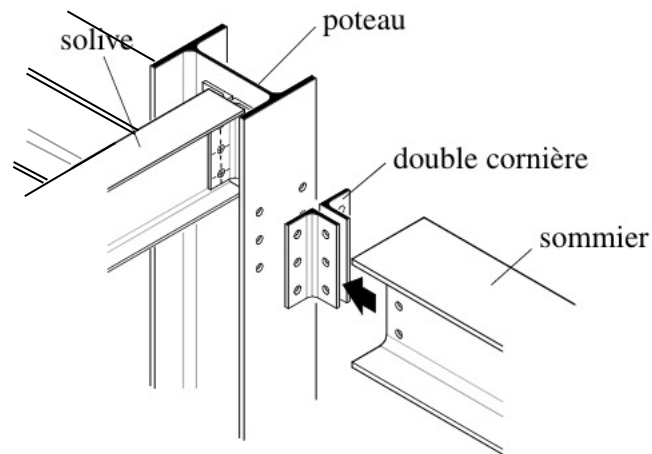
Fig. 4.2: Articulated (and braced) frames. Ex. of joints



(b) Poteaux continus



(c) Sommiers continus

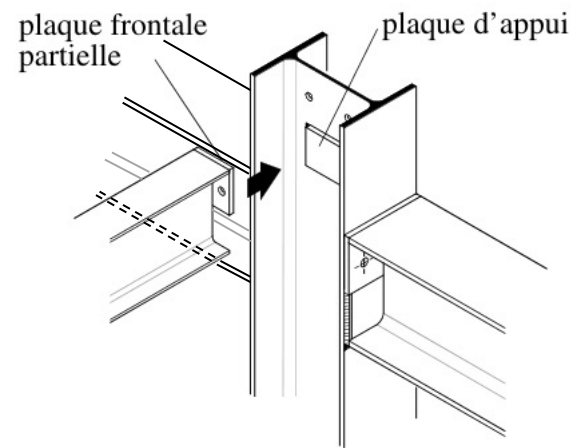


Vocabulary:

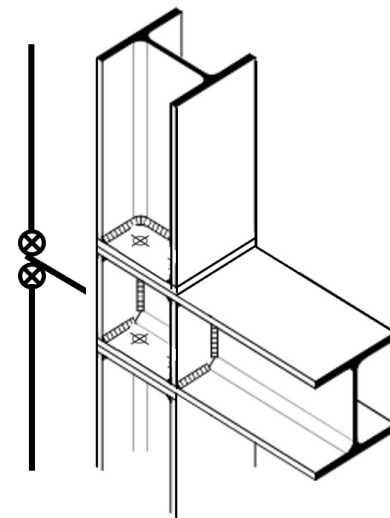
Double angle – Double cornière

Endplate – Plaque frontale

Seat plate – Plaque d'appui

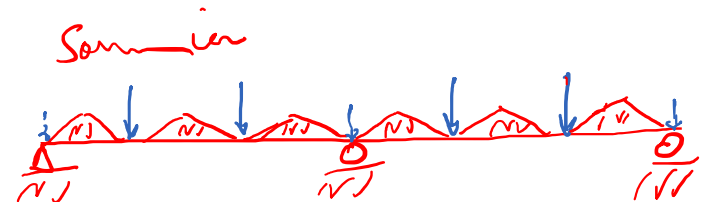
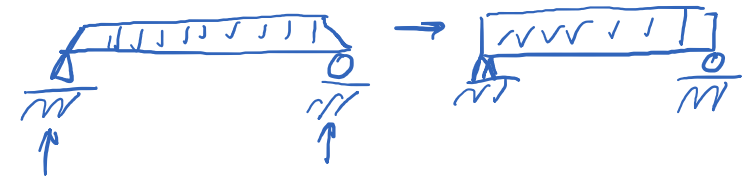
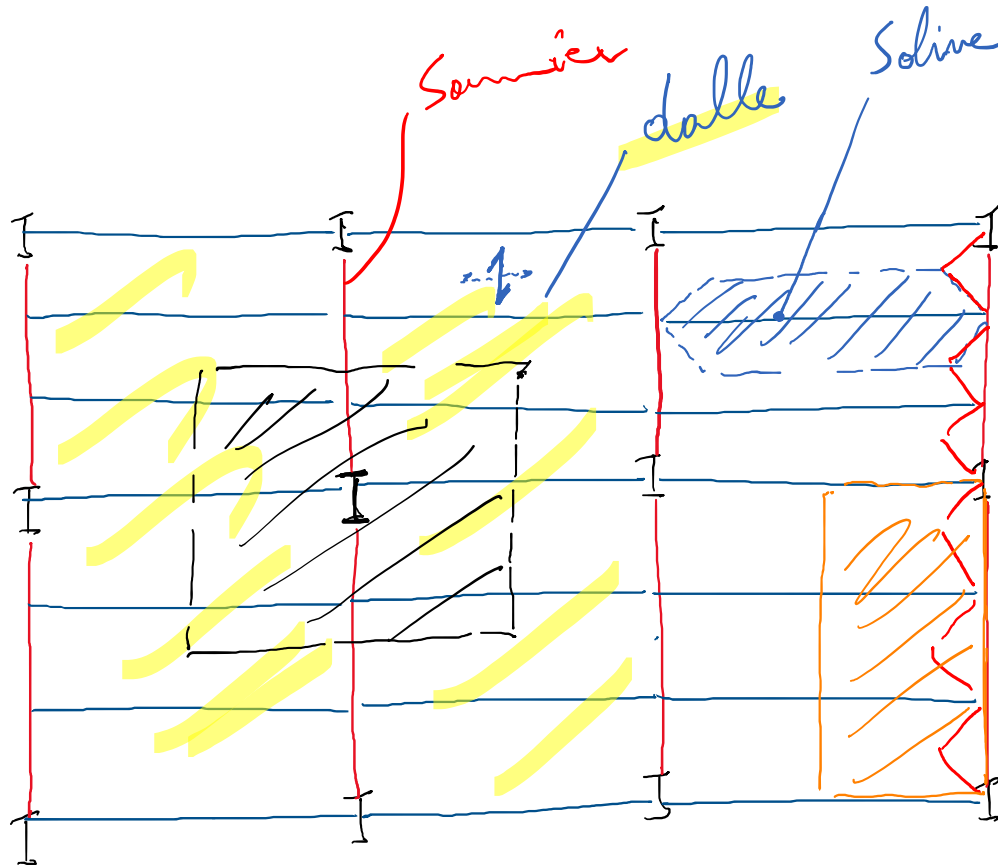


○ Hinges to
⊗ semi-rigid joints



Flooring, distribution for vertical load path to foundation (influence surfaces)

Plan view of a floor:

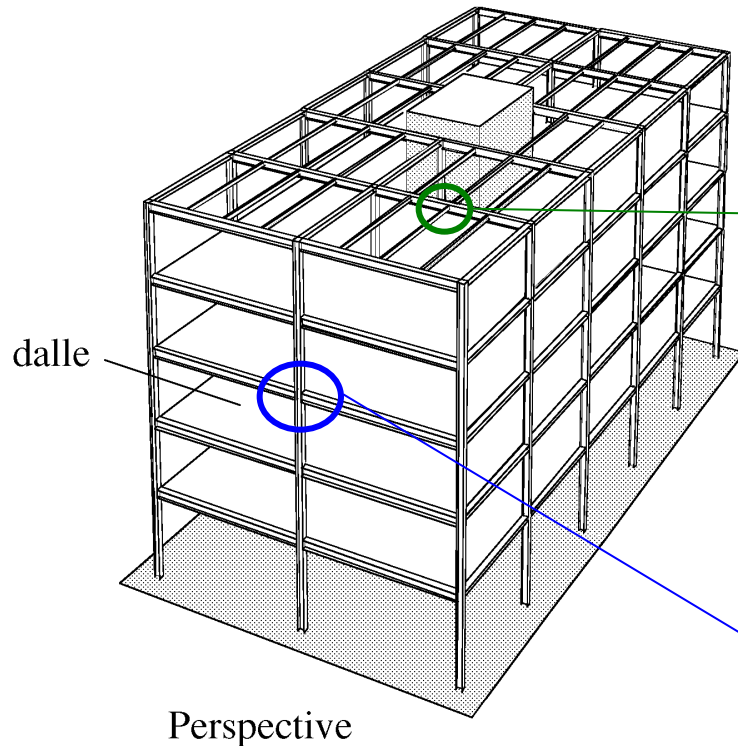


Simpl.

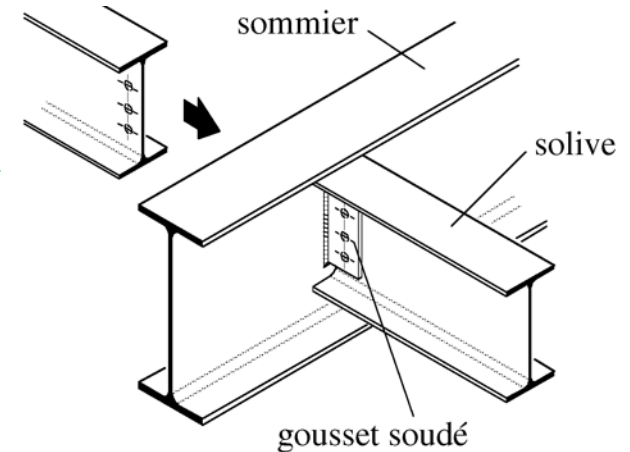


ex. 6.9.3 TGC 11

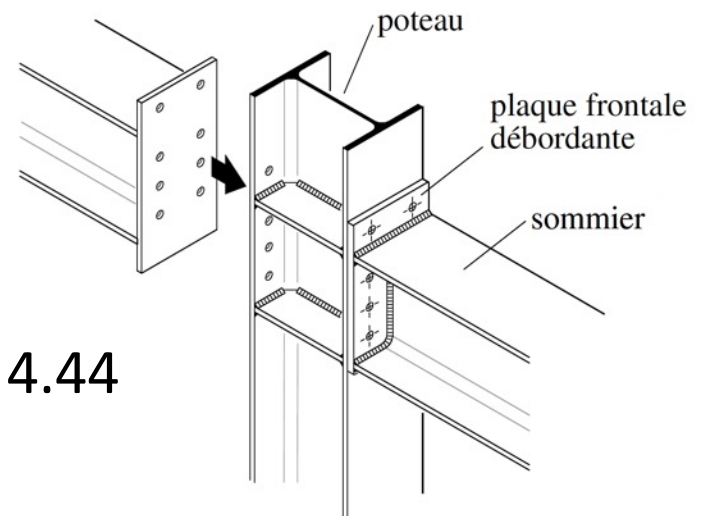
Other examples of joints between elements



Articulated secondary beams
(with bolted fins)



Rigid column-beam joint
(second. beam not shown)

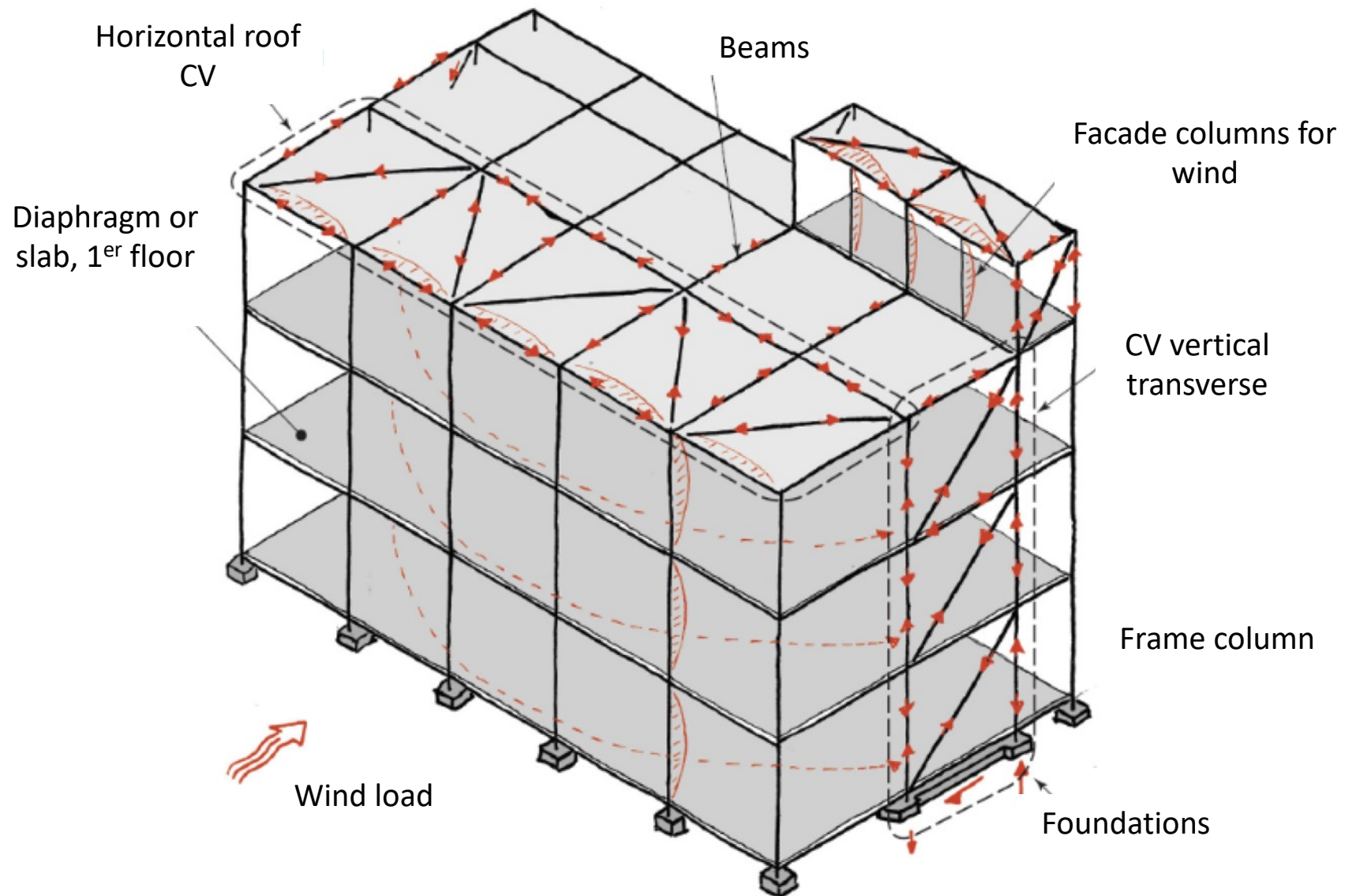


Joints ?

Hinged, figs 4.41, 4.43 and 4.44

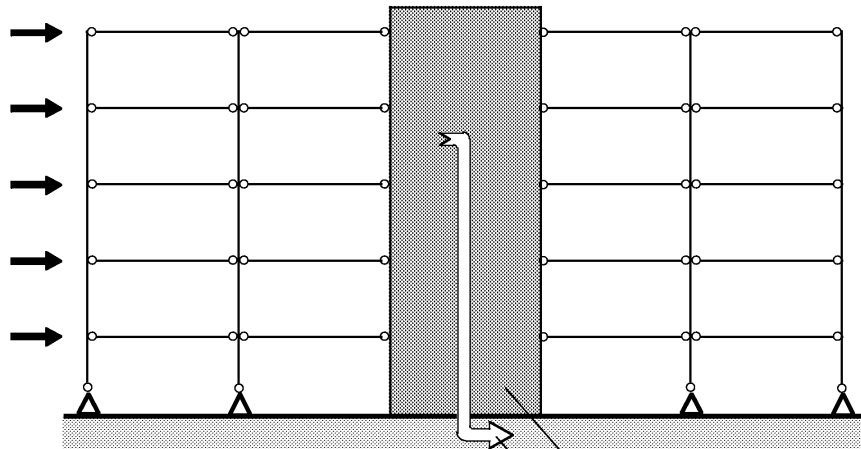
Rigid, figs. 4.42 and 4.45

Building: transmission of horizontal forces from façade to foundation



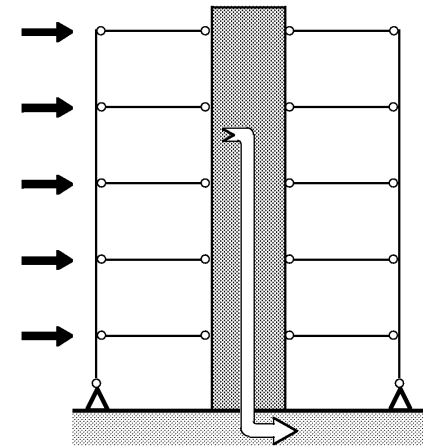
Source: IStructE guidance, www.istructe.org/

2) Fig. 4.3: (Articulated) frames with central core

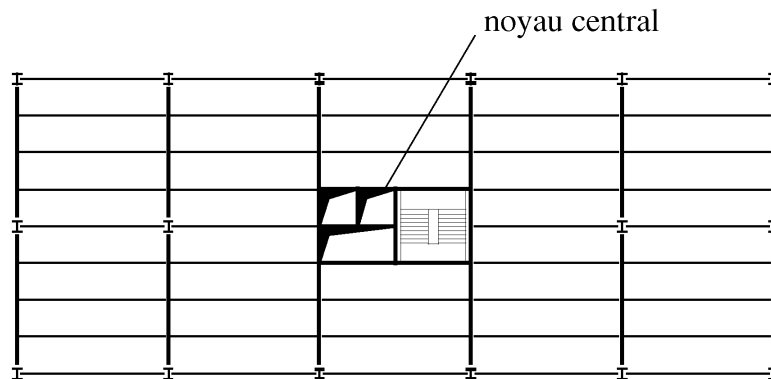


Coupe longitudinale

noyau central
cheminement des
forces horizontales

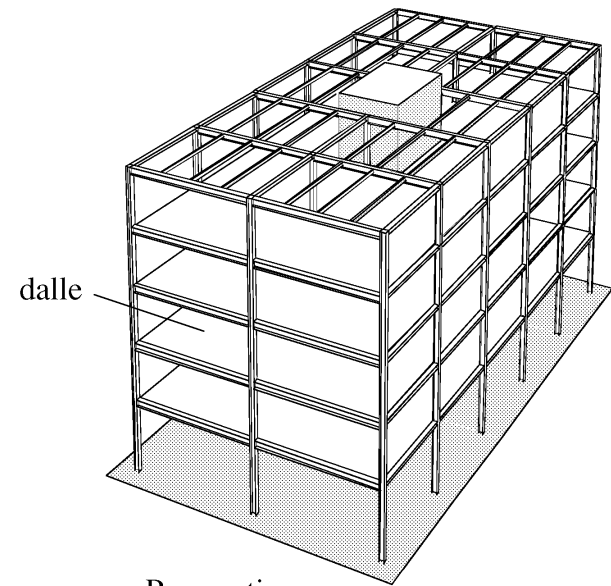


Coupe transversale



Plan

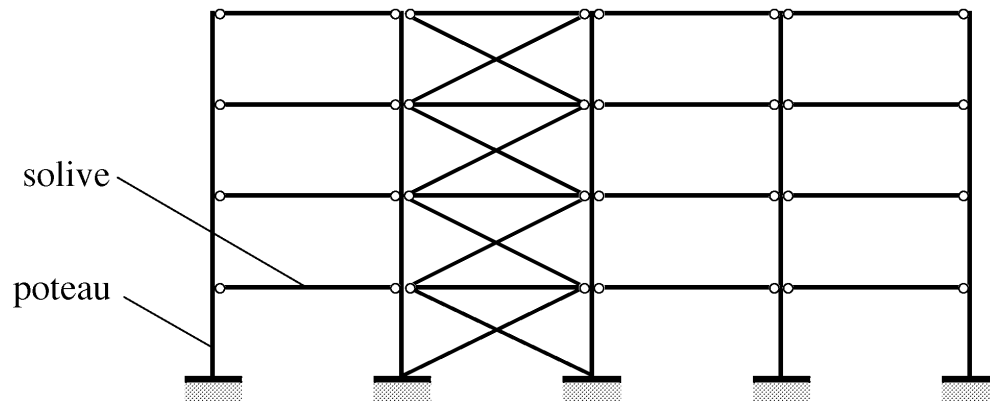
noyau central



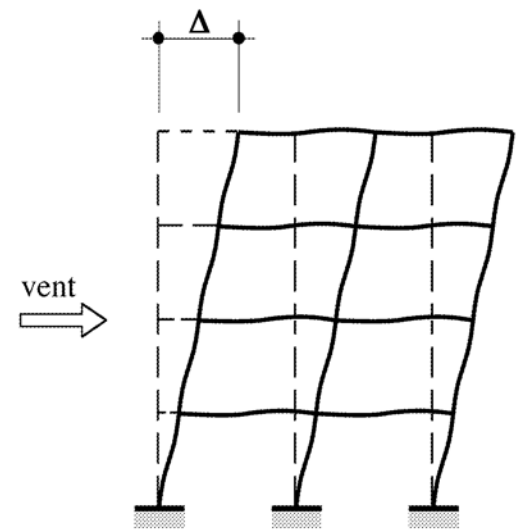
Perspective

Joints between steel & concrete core ? Figs. 4.48 to 4.52

Fig. 4.8: Rigid (or combined) frame construction



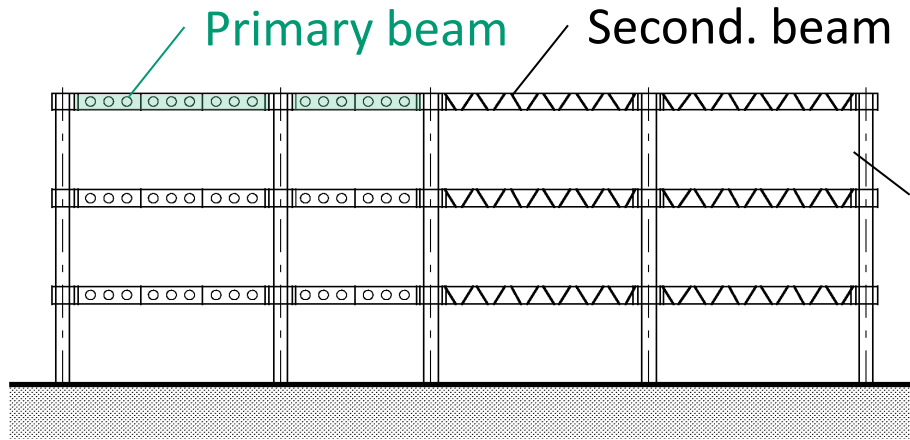
Coupe longitudinale



Coupe transversale

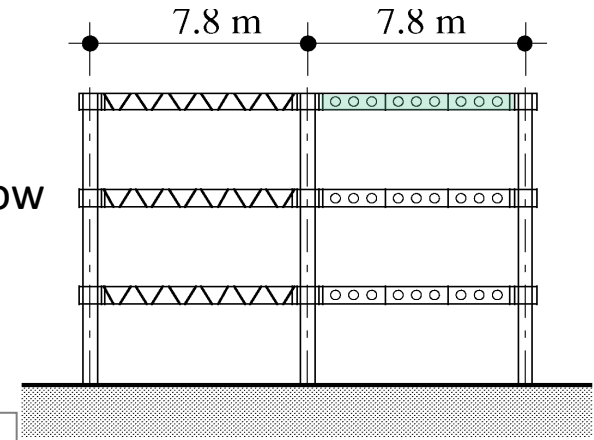
- ✓ Statically indeterminate, reduced element dim., robust system
- ✓ Larger usable area and flexibility (no core anymore)
- x Larger horizontal displacements (ok for buildings ≤ 5 storeys)
- x Complicated rigid joints
- x More sensitive to manufacturing tolerances
- x Columns more sensitive to 2^{ème} order, more bending
- x Generally more expensive \$\$, especially if fully rigid

Fig 4.9: Rigid frame structures, CROCS building example

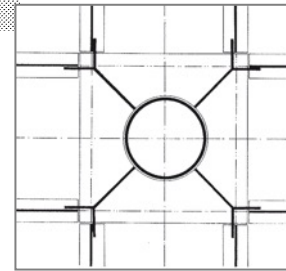


Coupe longitudinale A-A

Continuous columns
(circular hollow tubes), with mushrooms



Coupe transversale B-B

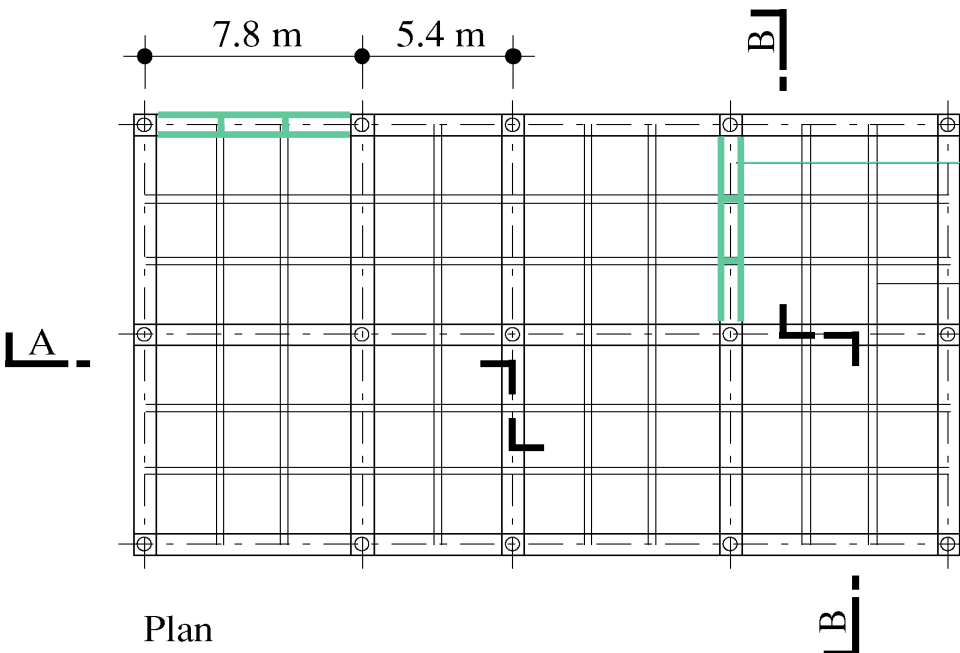


Primary beam combine 2 castellated I-beams with links where 2nd beams connect

Secondary beam, lattice girder

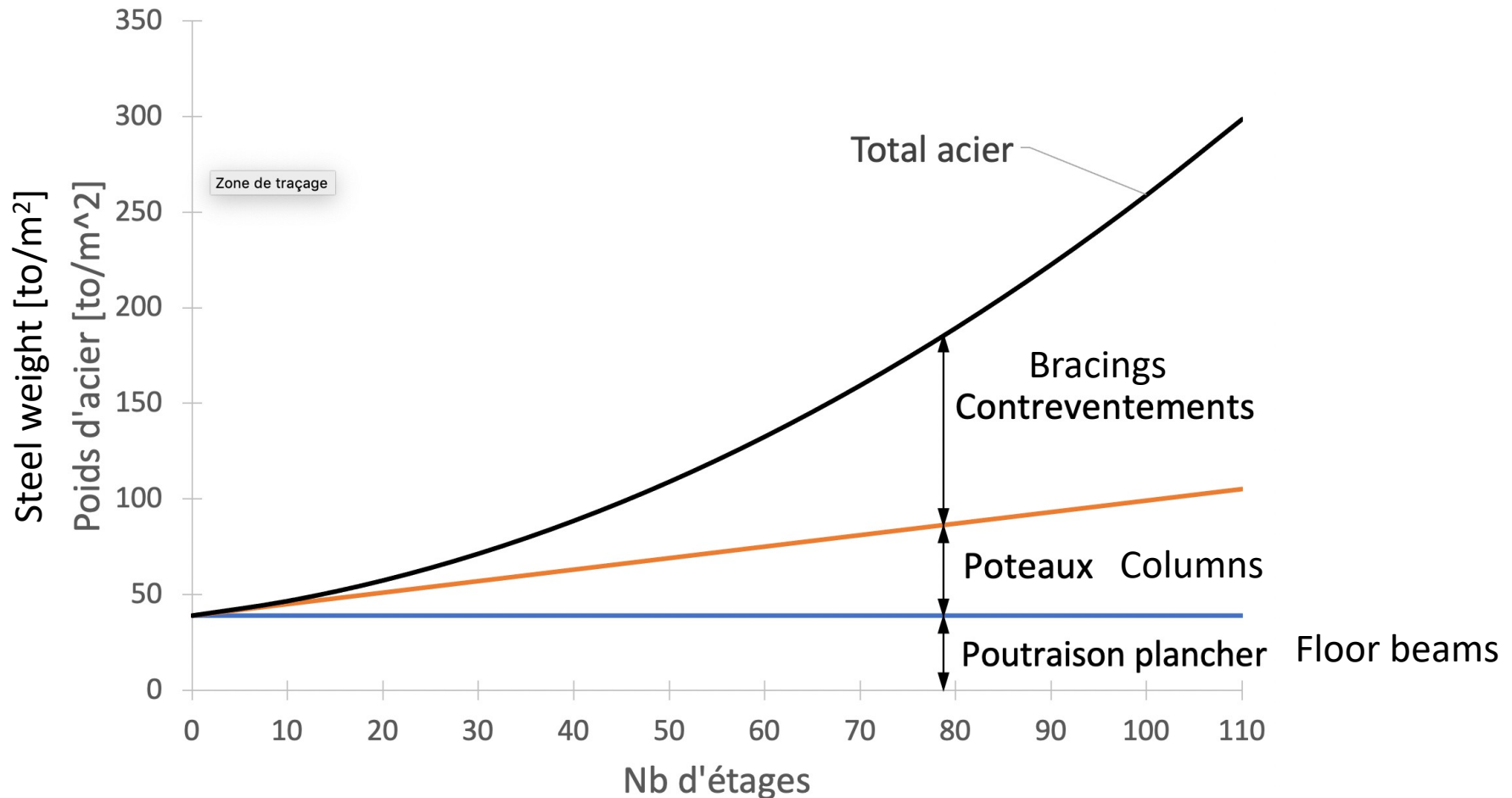
CROCS: centre de rationalisation et d'organisation scolaires. Number of schools built in the 1970s.

- + Speed of execution, many configurations
- In the end, flexibility yes, but expensive transformations
- Waterproofing and insulation problems



Plan

The challenge of high-rise buildings



- ✓ Wind loads are becoming increasingly important
- ✓ Involve all elements in horizontal force recovery

Summary of load-bearing systems for multi-storey buildings

Nb floors: ≤ 30



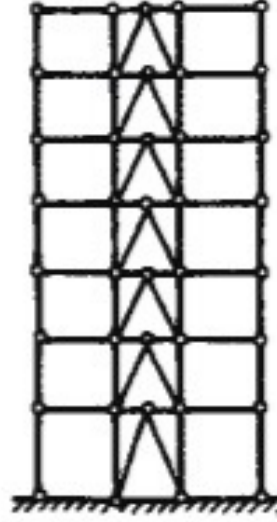
Regular,
frames
rigid

≤ 35

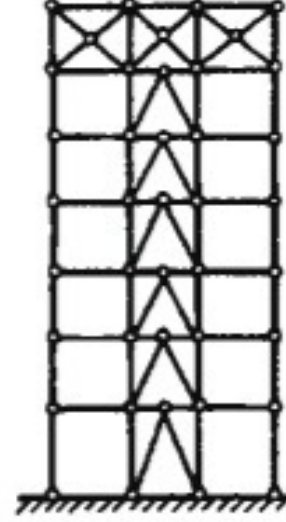


Varies according to
floors:
- Rigid frames
- Articulated
- Top floor

≤ 45

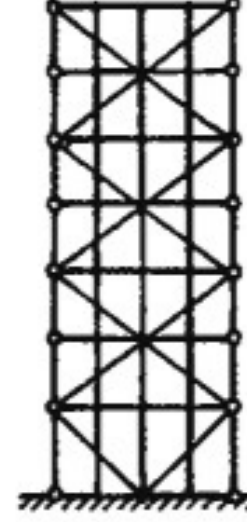


With central
steel core (and
hinged frames)

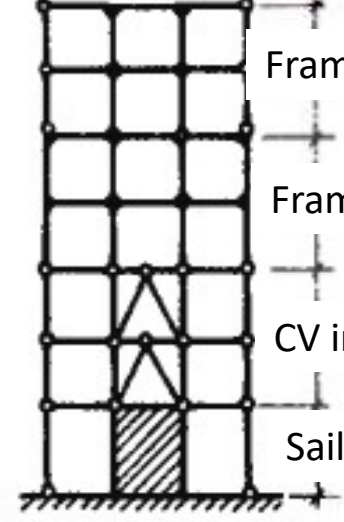


Plus a
stiffening
roof floor
(cap)

up to 85 (100 or more, multitube)



Peripheral
Tube
(shown
with X-bracings)



Combination
of systems

Frame type 2

Frame type 1

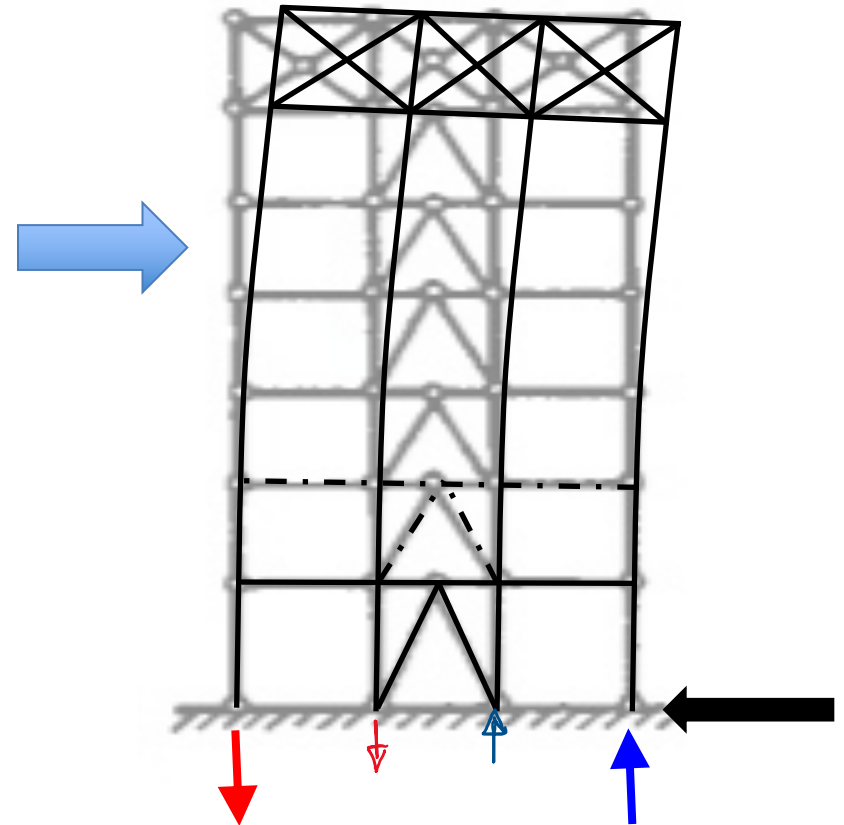
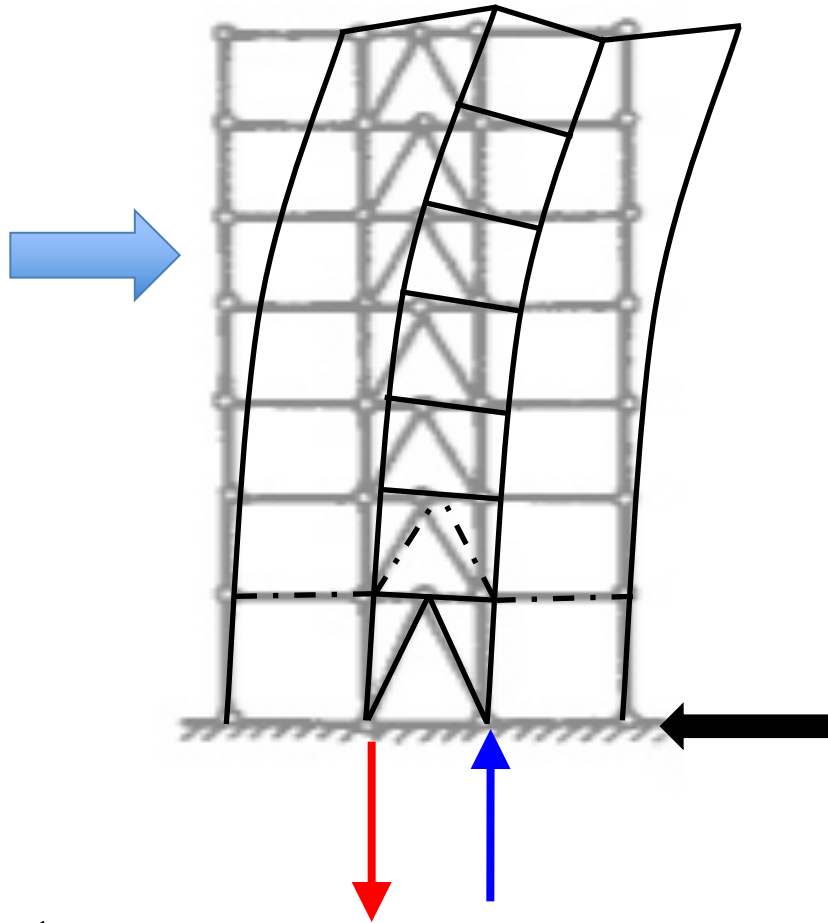
CV in K

Sail

Source: Extract from the book Industriestahlbauwerke

+ Particular, hybrid systems: some examples in TGC 11 § 4.5

Taller buildings: e.g. add a cap or stiffening belts



- ✓ Limiting horizontal movements
- ✓ Participation of external columns to horizontal loads transfer
- ✓ Service/mechanical floor(s) (also an inconvenience, add height)

Breakdown of costs for a typical multi-storey office building

- Foundations 5 to 15%
- Superstructure and floors 10 to 12%
- Façades and roof 15 to 25%
- Services (mechanical, electrical, lifts) 20-25%
- CVSE (water and sanitation) 5 to 8%
- finishing work, partitions and adjustments 10 to 20%
- Site management (preparation, excavations) 12 to 18%

Source: www.steelconstruction.info

Pre-sizing of elements (buildings)

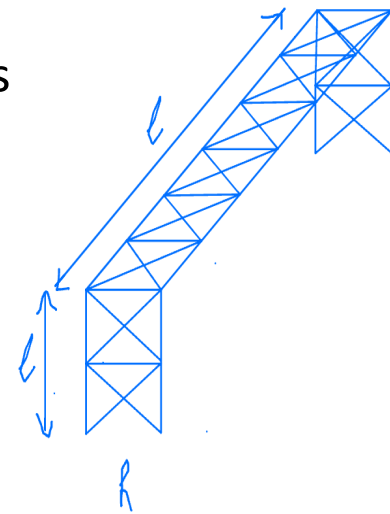
From experience, from TGC 11 § 3.2.3, § 5.3.3 and Appendix chap. 6: rules of thumb for pre-sizing elements

Spans and spacings:

- Compromise between adaptability (large spans) and economy (small spans)
- Frame spans: typically between 6 and 15 m (standard hot-rolled sections), but possible up to 30 m
- Main beam spacing (= 2nd beam spans): typically between 6 and 9 m, but possible up to 12 to 15 m (lattice girders)
- 2nd beam/purlin spans: typically between 6 and 9 m
- Purlin spacing: from 1 m (fibre cement corrugated roofing sheets without substructure) to 4 m (profiled steel sheets or decking).

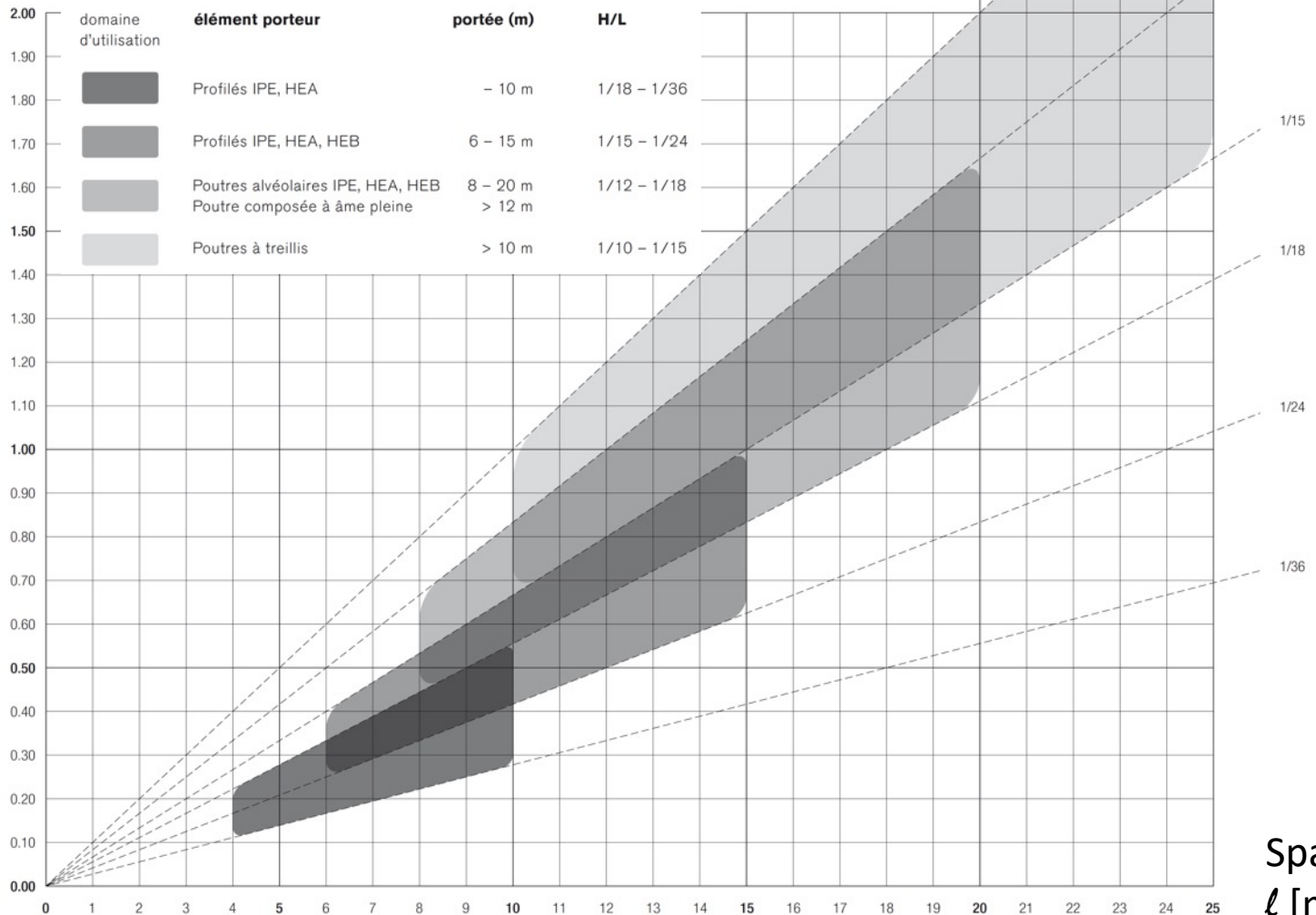
Slenderness:

- Composite slab with profiled steel decking: $d \cong \ell/32$ (d : static height)
- Hot-rolled purlin: $h \cong \ell/30$ (h : static height)
- Hot-rolled main beam: $h \cong \ell/20$ to $\ell/25$
- Hot-rolled section column: HE section with slenderness $\lambda_k \leq 50$
- Bracing, equiv. to lattice with low slenderness, if possible: $h \cong \ell/5$ to $h \cong \ell/8$



Pre-dimensioning of steel beams

Element height
 h [m]



Span
 l [m]

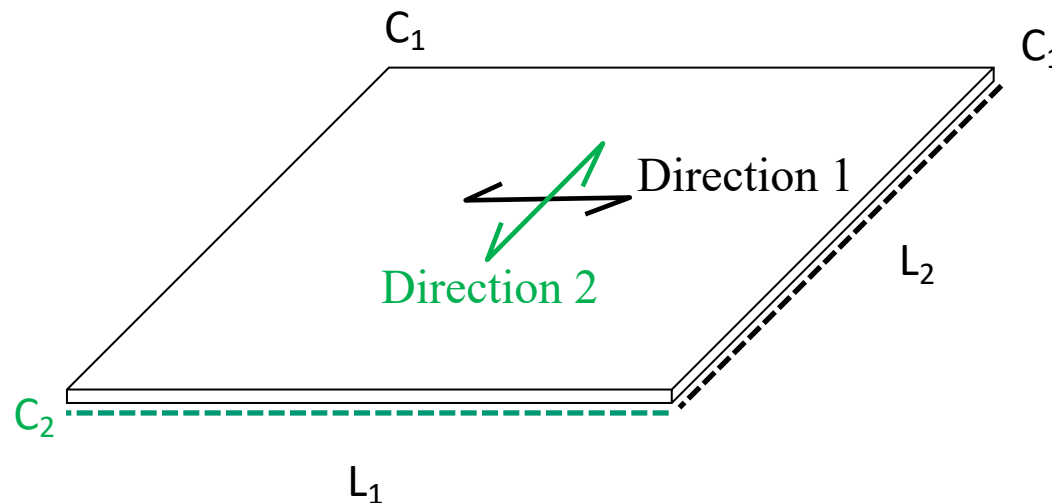
(Source: M. Dietrich: Ecole d'ingénieurs Burgdorf, 1990)

Slabs: Summary of the strip method

L / L_{12}	C / C_{21}				
	0,2	0,4	1,0	2,5	5,0
1,0	0,17	0,29	0,5	0,71	0,83
1,2	0,29	0,45	0,67	0,84	0,91
1,5	0,5	0,67	0,84	0,93	0,96
2,0	0,76	0,86	0,94	0,98	0,99

Same supp. conditions, the shorter (dir. 2) takes 84%

C_1 , C_2 : stiffness factors for support lines (e.g. 1/384 simple support, 5/384 rigidly fixed)

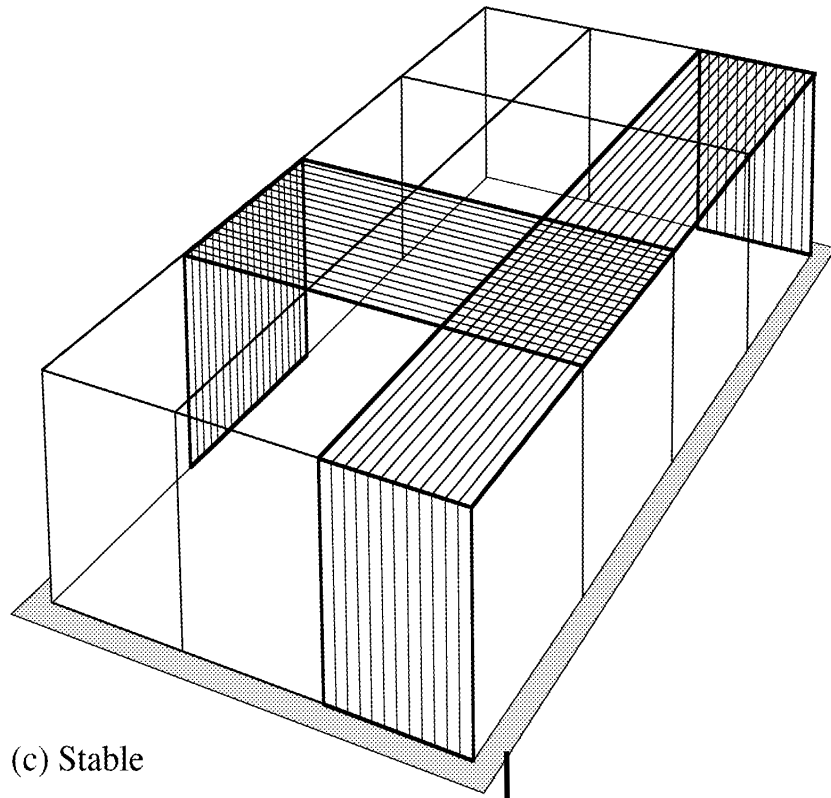


APPENDICES

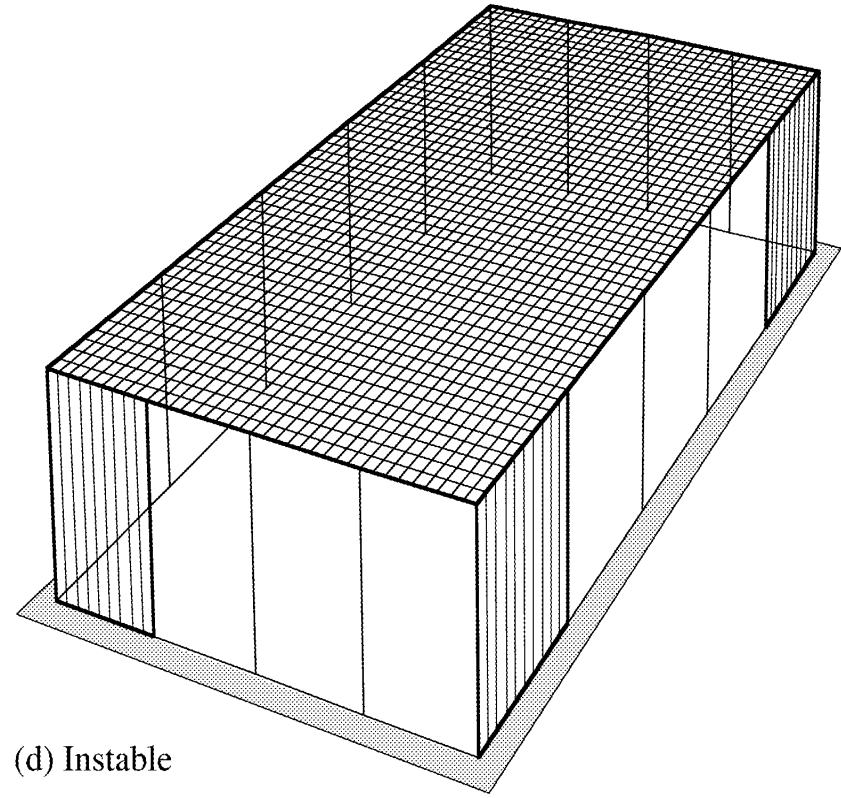
Recall of stabilization

Examples of buildings and
towers with tube structures

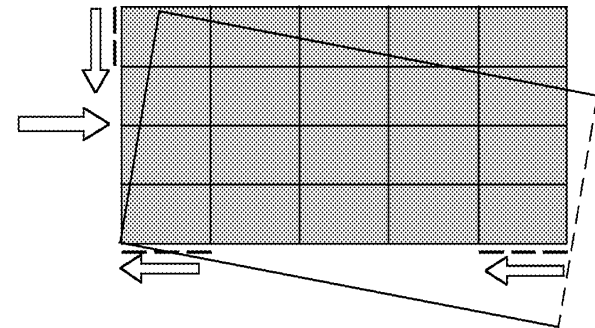
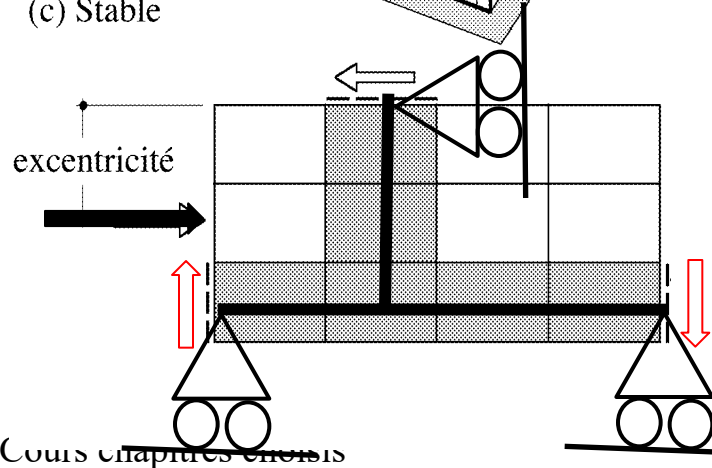
Reminder: bracing arrangement (TGC 11, fig. 3.19)



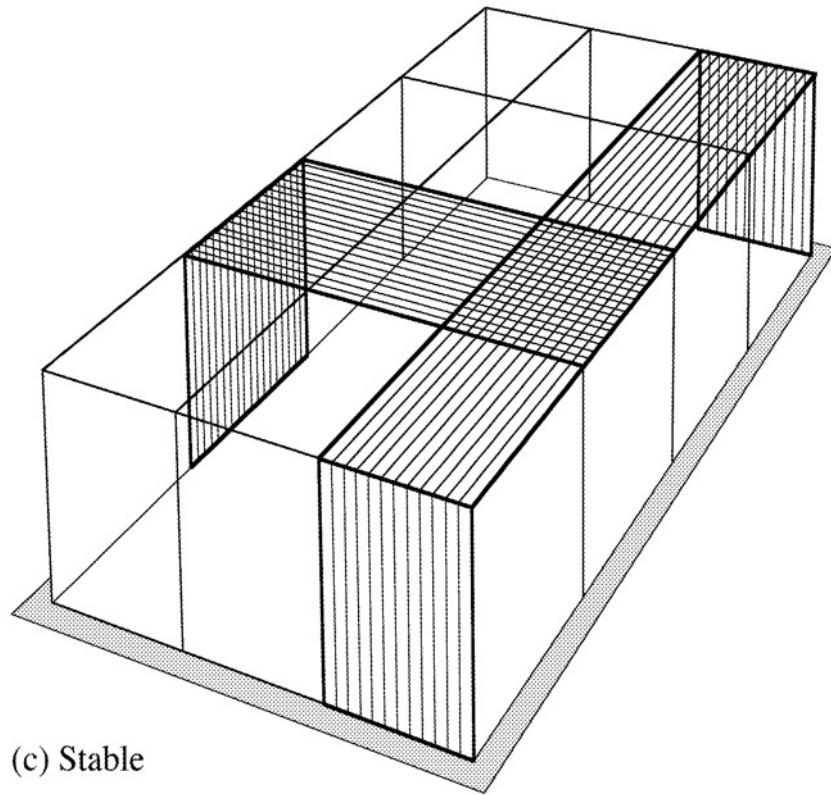
(c) Stable



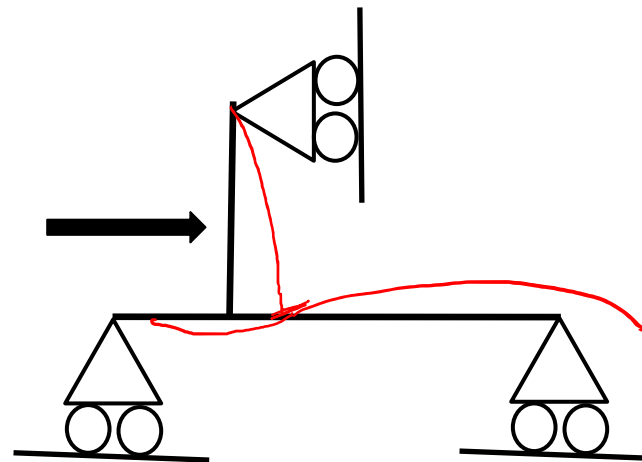
(d) Instable



Reminder: bracing layout (TGC 11, fig. 3.19)

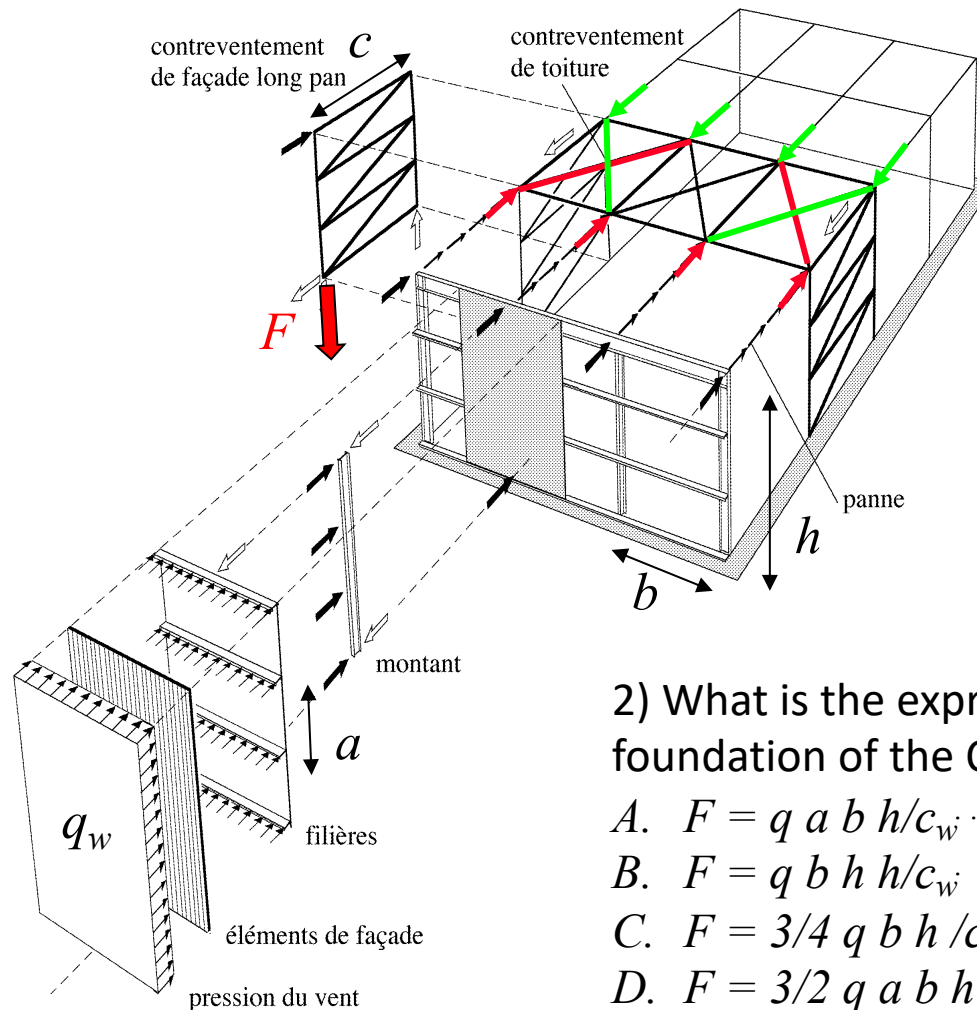


(c) Stable



Reminder: horizontal force path (TGC 11, fig. 3.20)

Only consider bars in tension



2) What is the expression of force in the foundation of the CV?

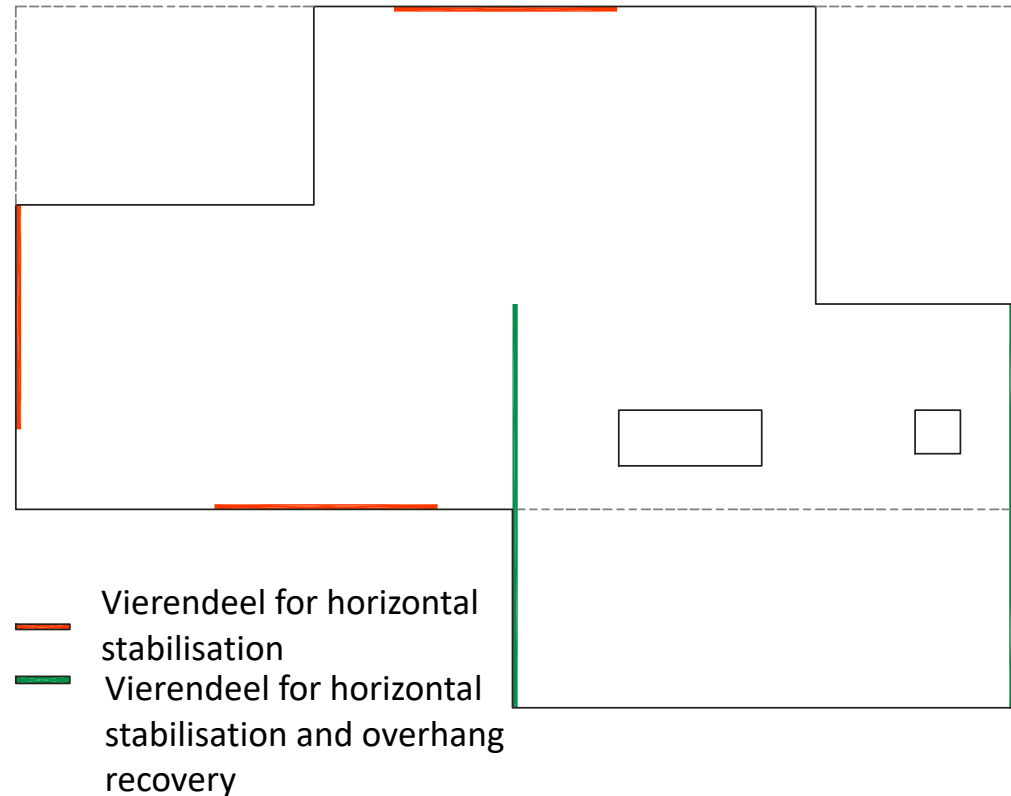
- A. $F = q a b h / c_w \dots$
- B. $F = q b h h / c_w \dots$
- C. $F = 3/4 q b h / c_w \dots^2$
- D. $F = 3/2 q a b h / c_w \dots^2$

Rigid frame structures, example Maladière school, Ne



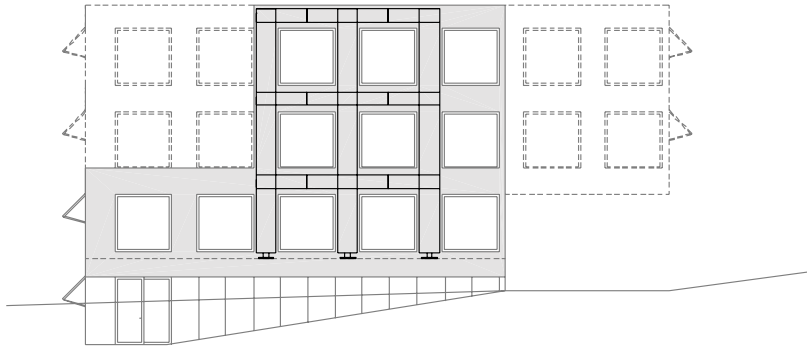
Sources: arch. Andrea Bassi / ing.
Guscetti & Tournier, 2006

Horizontal stability of the structure, plan view

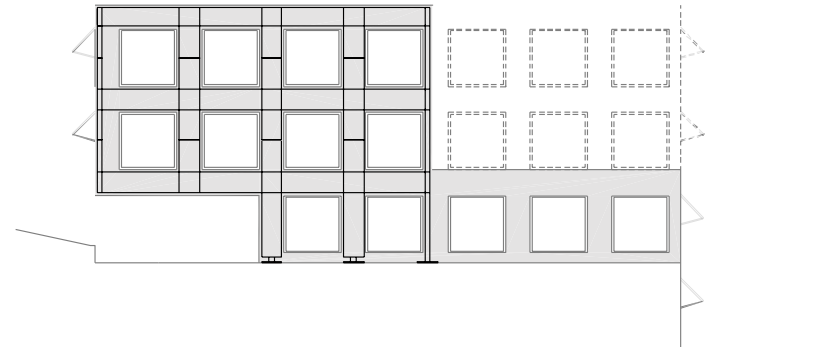


— Vierendeel for horizontal
stabilisation
— Vierendeel for horizontal
stabilisation and overhang
recovery

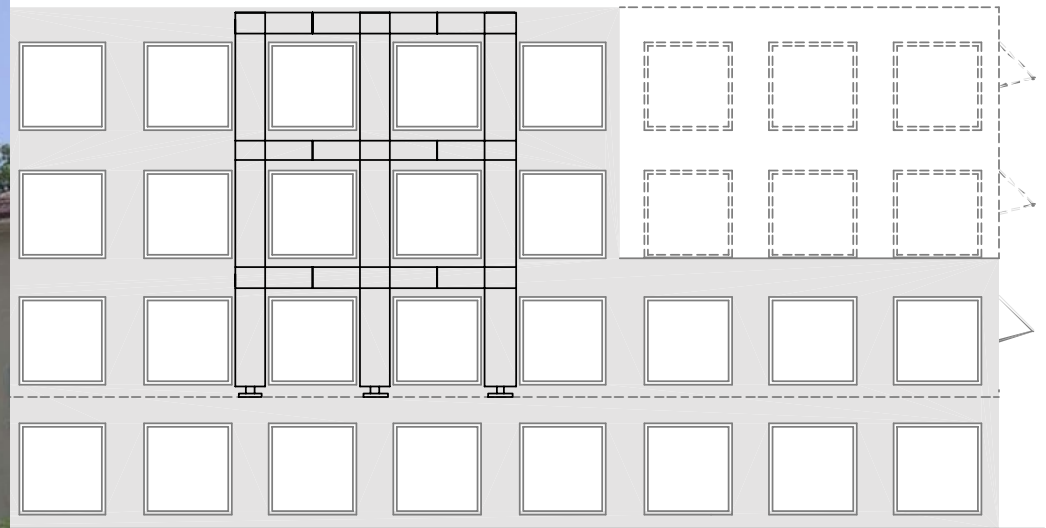
Rigid frame structures, example Maladière school, Ne



South façade

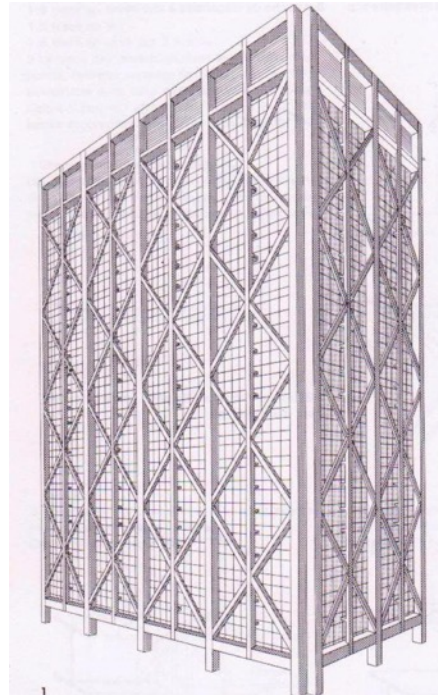


North facade



West façade

5) Tall buildings. Tubular structures with reinforced caissons



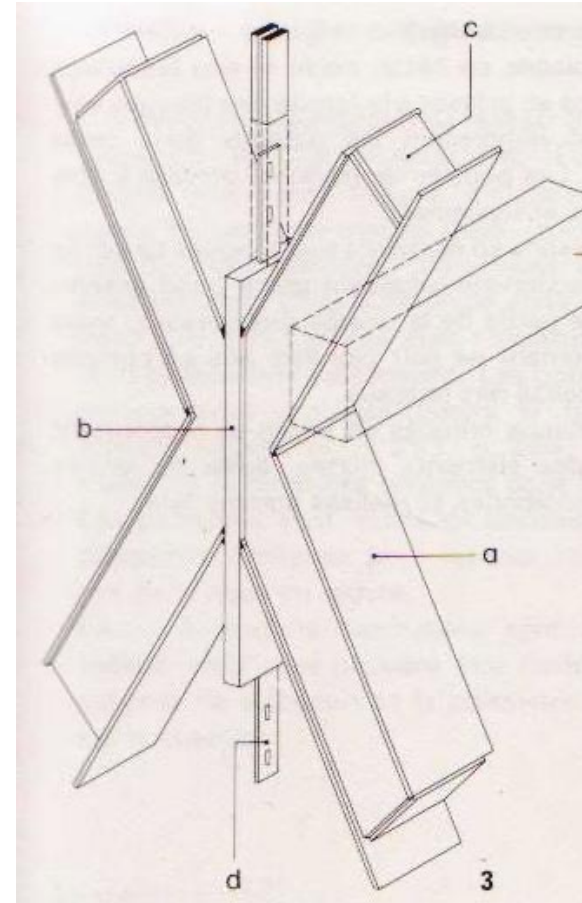
Alcoa administration building, San Francisco

- Heavy lattice structure: helps support vertical loads
- Also known as DIAGRID
- Diagonals are made of welded boxes interrupted at location of vertical gusset and butt-jointed further away
- Hangers are used to take the loads of the intermediate floors.

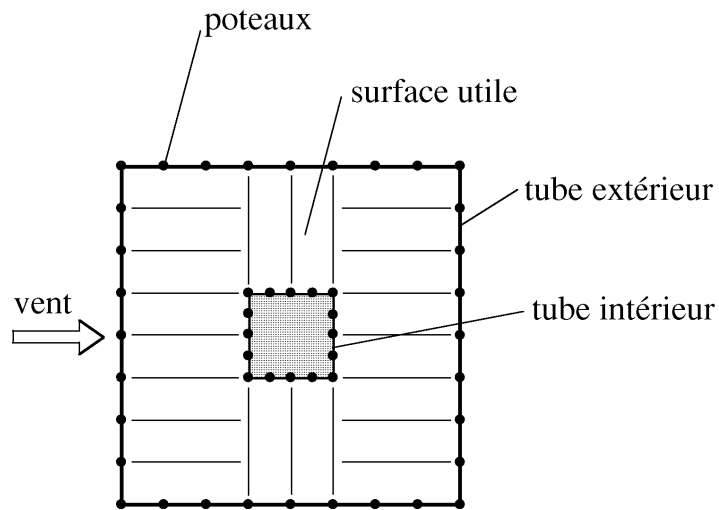
5) Tubular structures with reinforced caissons

Diagonals in welded boxes help to support vertical loads in addition to stiffening the building outer tube

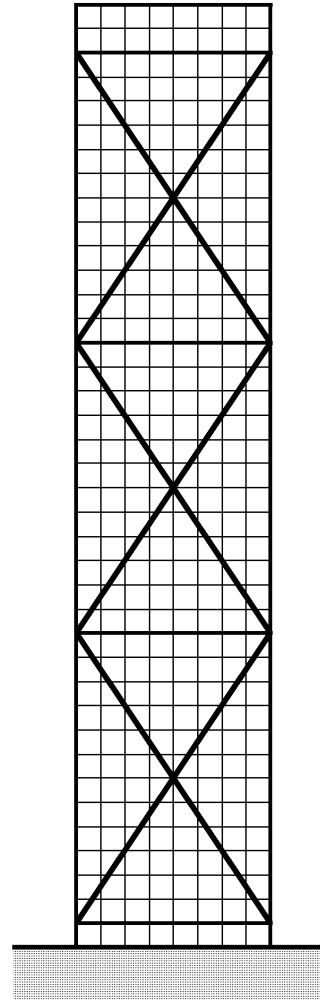
The hanger *d* supports loads of intermediate floors



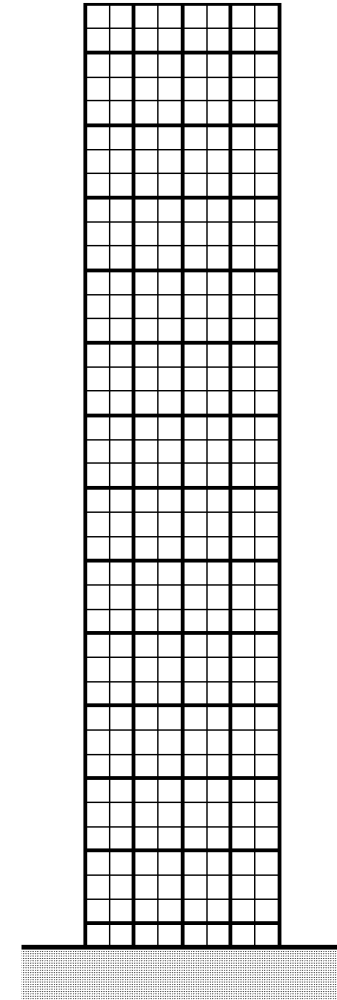
5) Tube structures \Leftrightarrow high-rise buildings (figs TGC 11)



(a) Plan de l'immeuble



(b) Façades avec contreventement en treillis



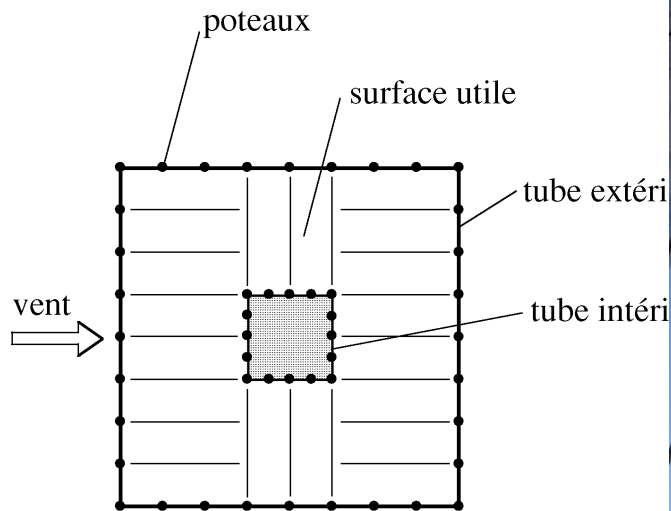
(c) Façades en cadres rigides

Fig. 4.12

John Hancock tower
arch. B. Graham, ing. F. Khan

World Trade center towers
M. Yamasaki, J. Skilling & R. Robertson

5) Tube structures \Leftrightarrow high-rise buildings



(a) Plan de l'immeuble



(b) Façades avec contreventement en treillis



(c) Façades en cadres rigides

5) Tube structures \Leftrightarrow high-rise buildings



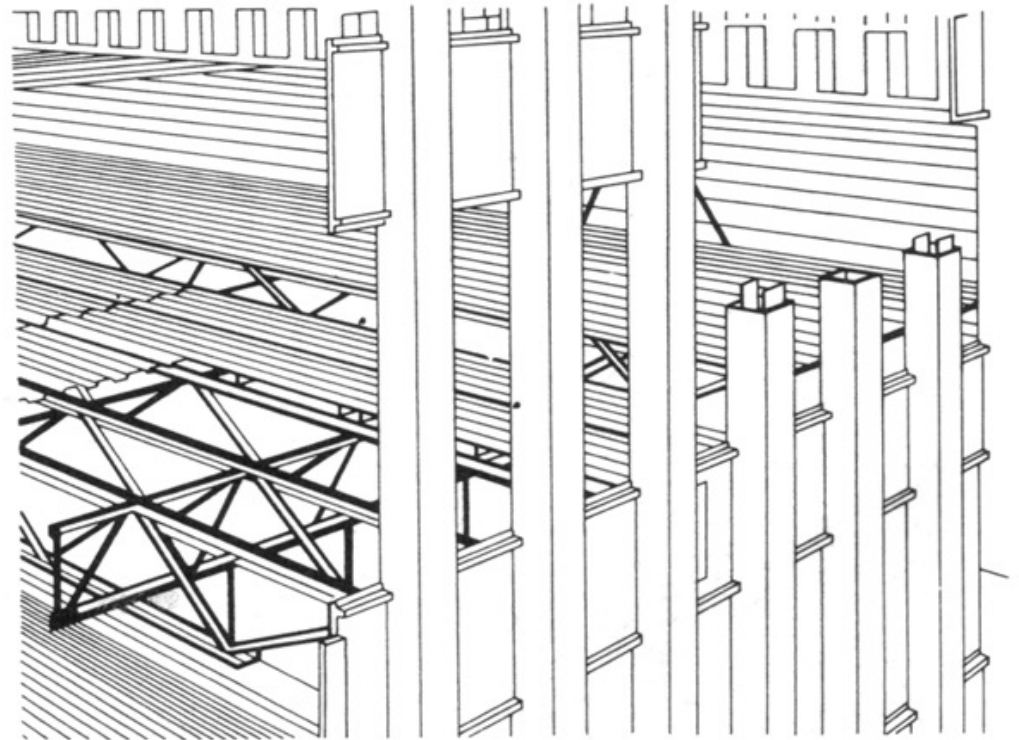
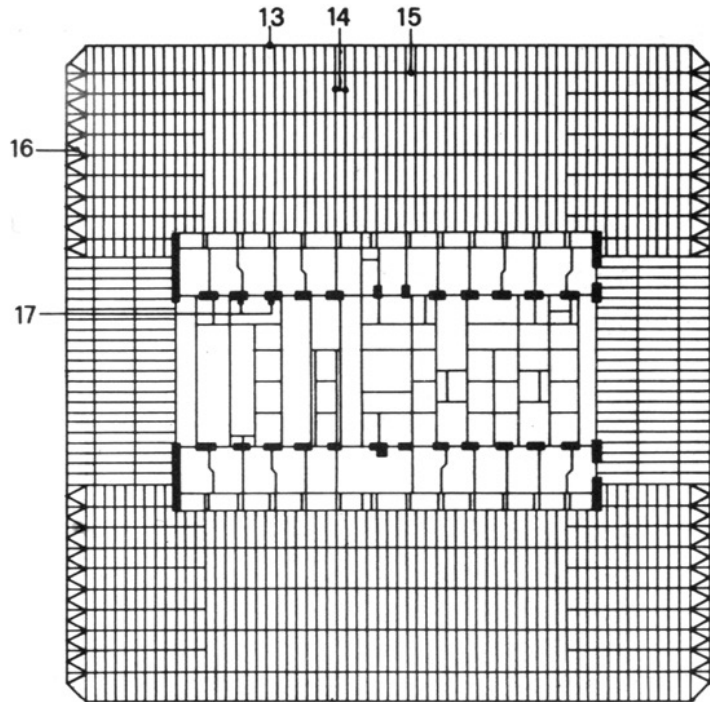
WTC:

Facade (40% of vertical loads)

Core (60% of vertical loads)

Horizontal loads on the outer tube

5) Tube structures ⇔ high-rise buildings

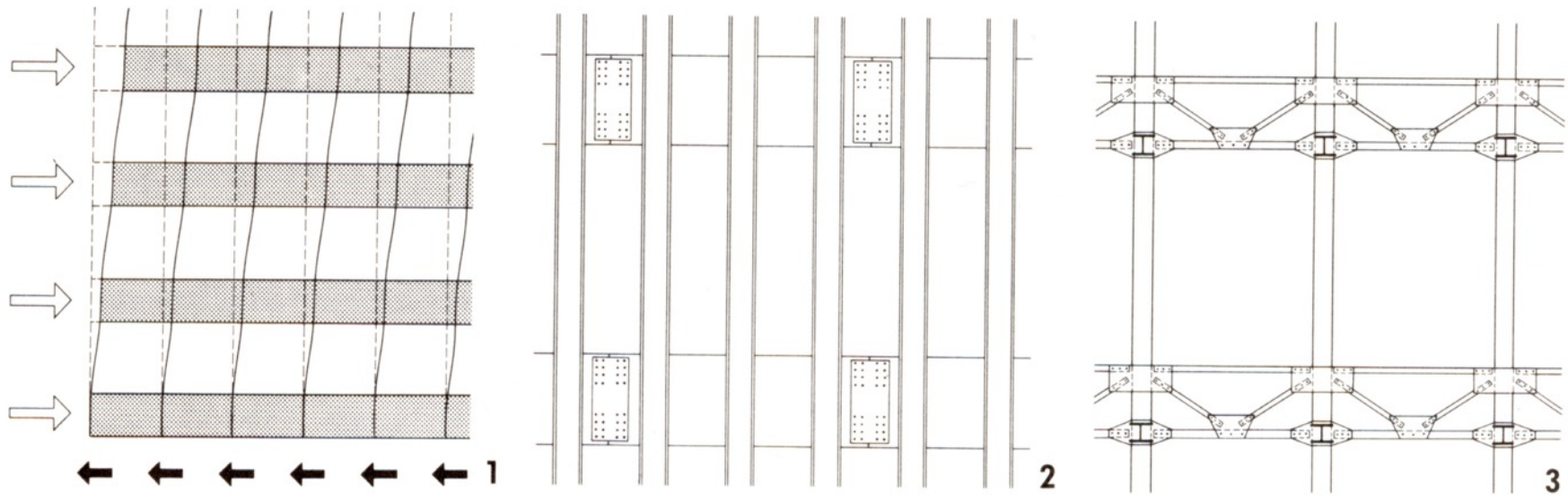


World Trade Center, New York

arch. M. Yamasaki & co., eng. J. Skilling & R. Robertson & co, 1973

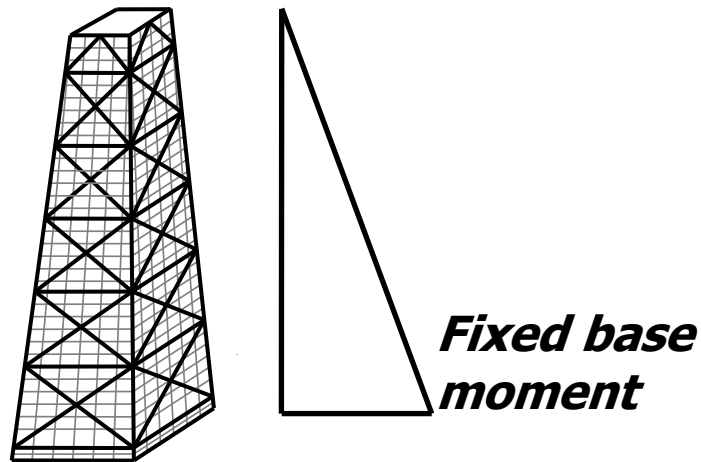
Facade system (or multiple Vierendeel)

Behind the façade parapets (sprandels) are rigid beams which, together with the external facade columns, form a multiframe in the plane of the façade.

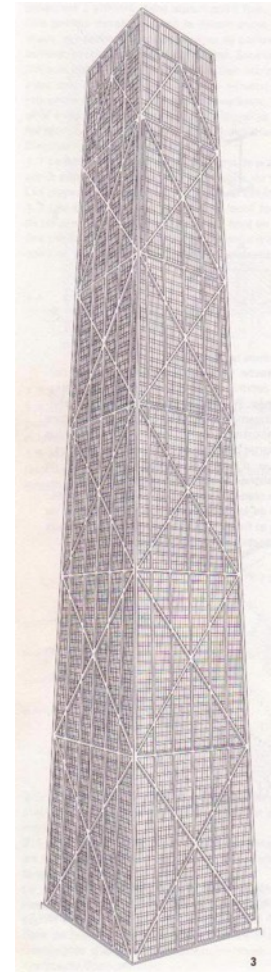


1. The façade beams (the same height as the parapet) are very rigid compared with the columns
2. Plated spandrel beams
3. Truss spandrel beams.

5) Tube structures \Leftrightarrow high-rise buildings



**Equivalent to a variable inertia beam
→ Larger cross-section at fixed base**

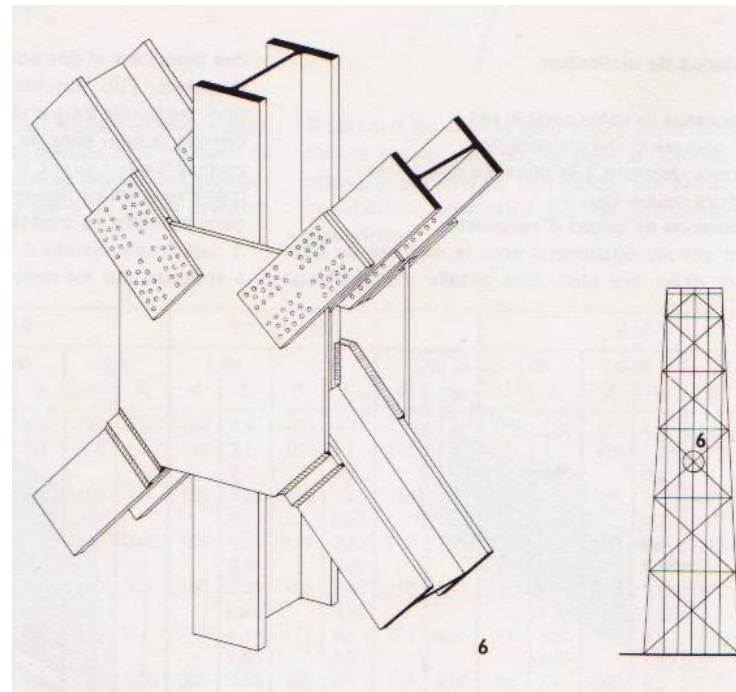


5) Tube structures \Leftrightarrow high-rise buildings

- John Hancock Center, Chicago
- Typical construction assembly

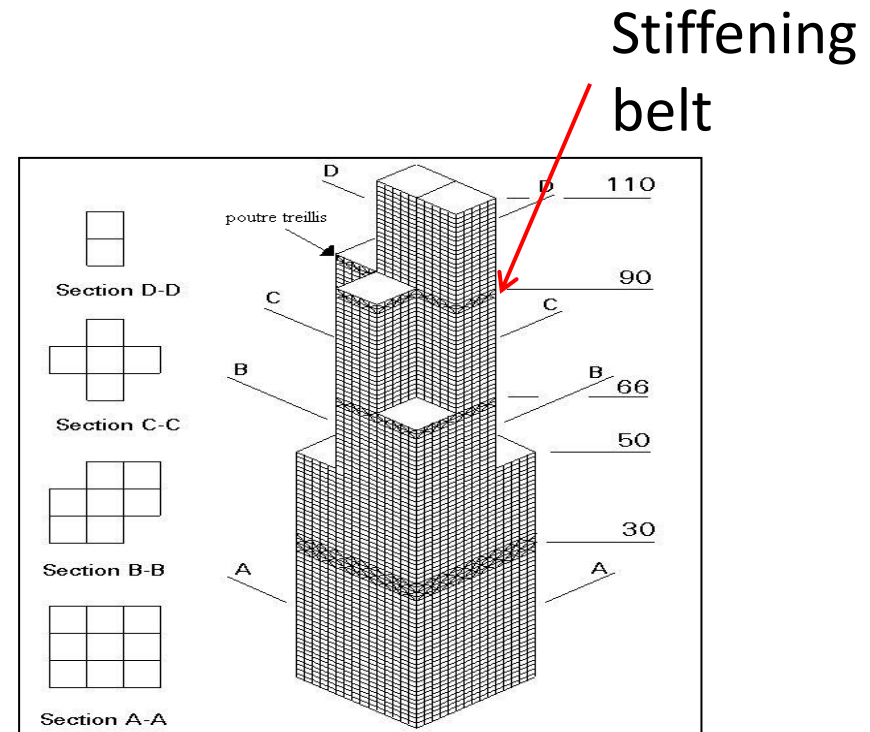
Heavy gussets (one storey high)

Reconstituted welded sections (huge beams and columns)

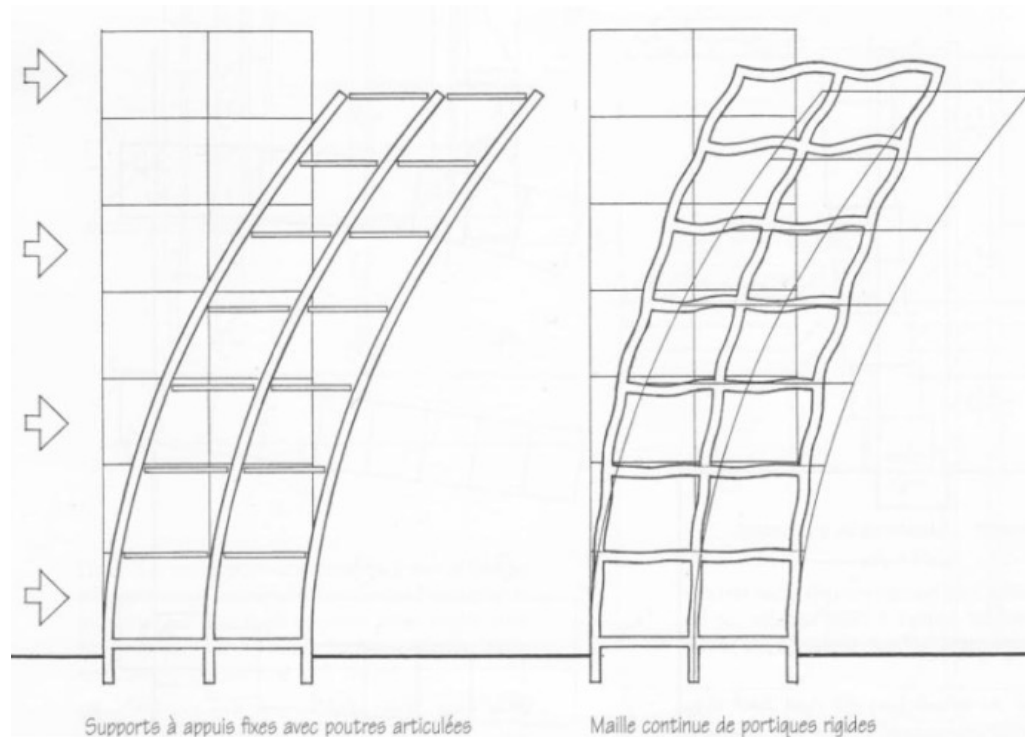


5) Tube structures \Leftrightarrow high-rise buildings

- John Caisson structures: tube bundles
- The Willis tower, Chicago



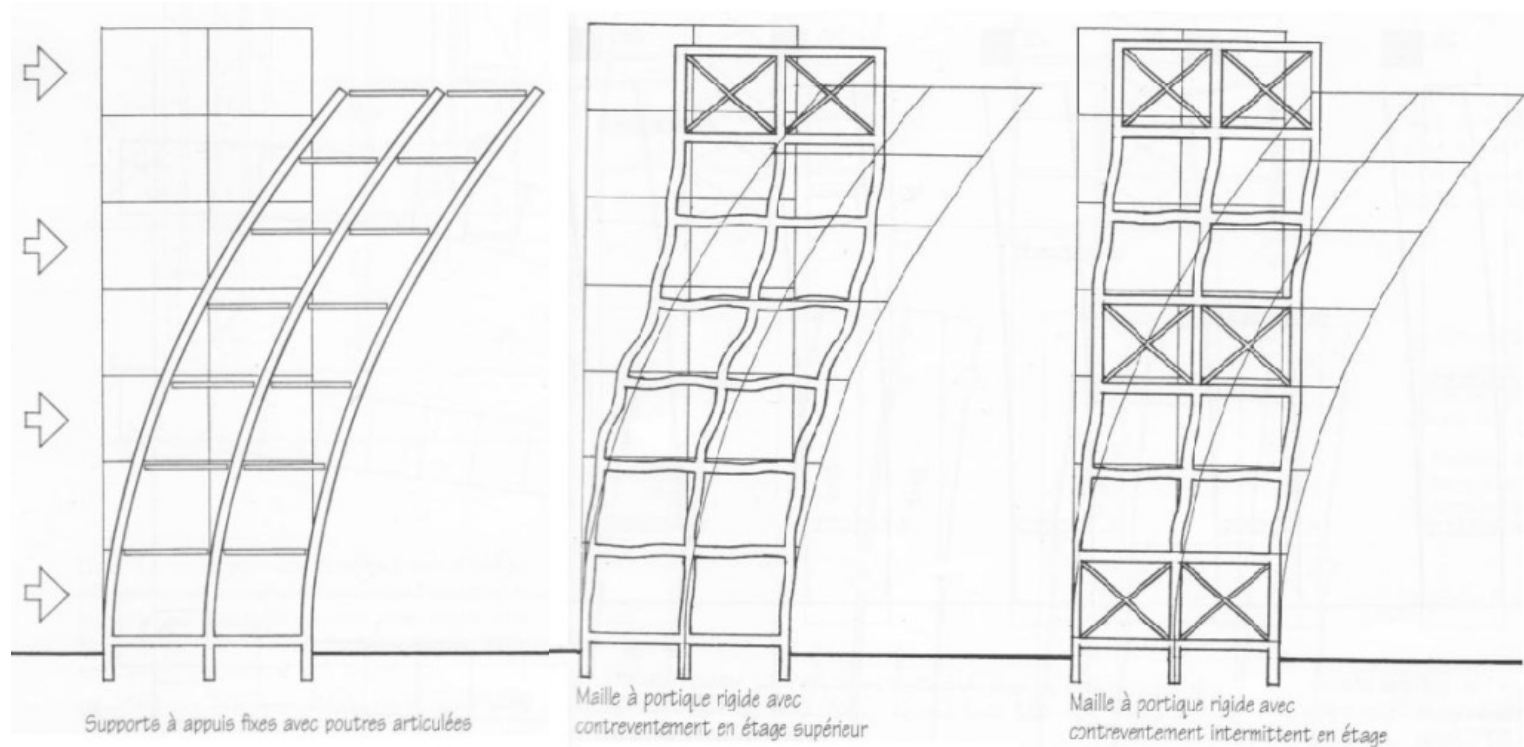
- Limit horizontal displacement of tall buildings



With articulated beams

Rigid mesh

■ Limit horizontal displacement of tall buildings



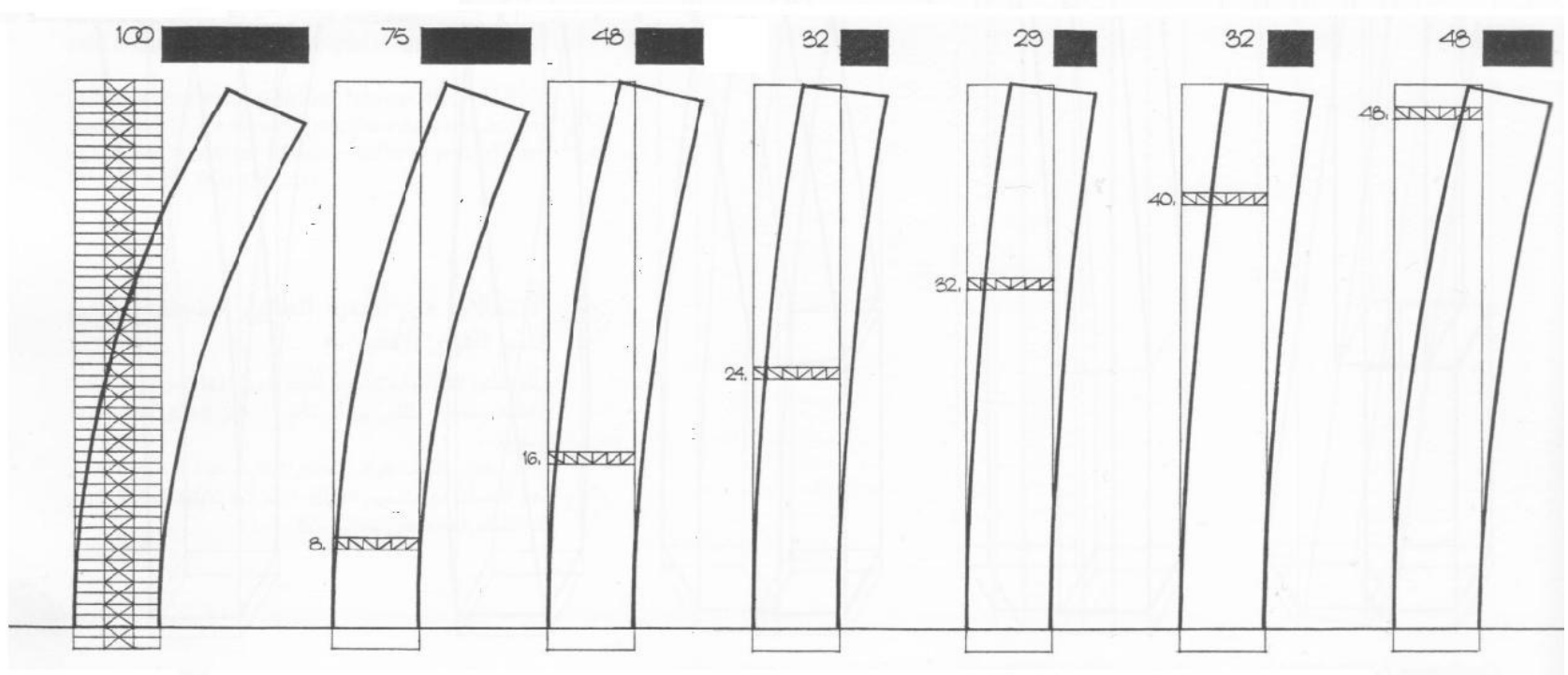
With articulated beams

Rigid mesh + 1 stiff.

Rigid mesh + 3
stiffening floors

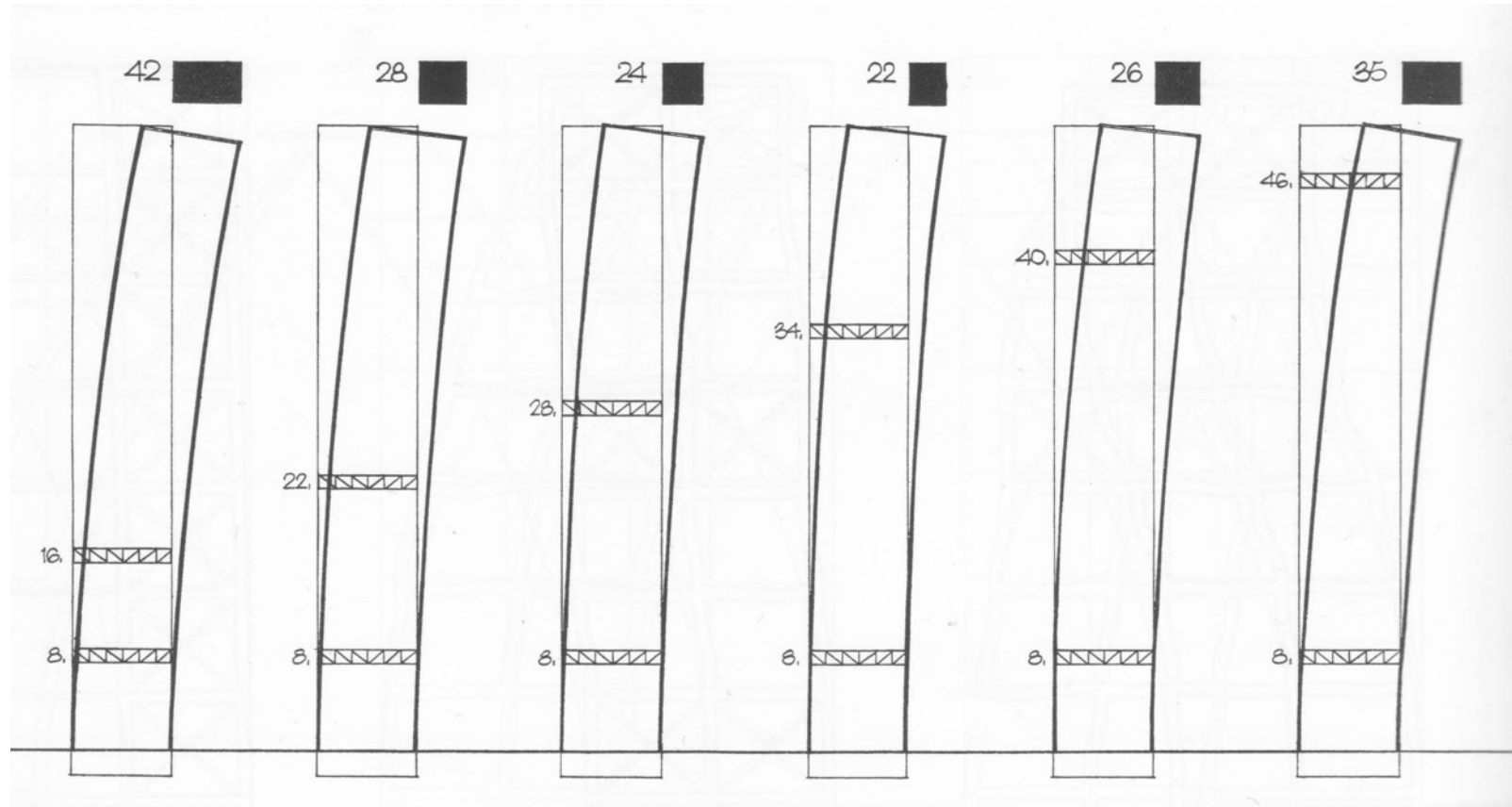
=> the console « beam » has become more rigid

- Decisive position → 1 stiffening belt



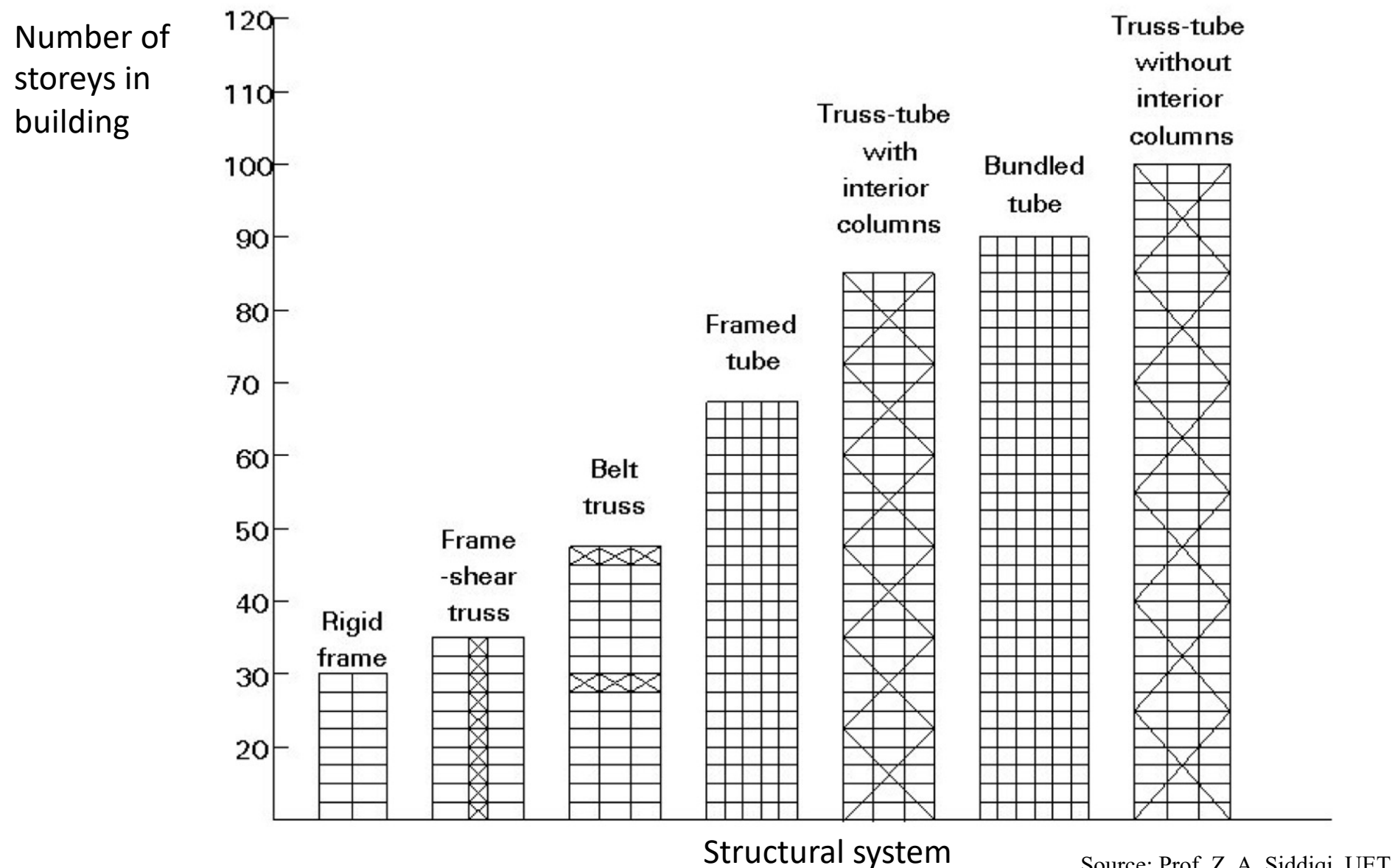
Ideally $3/5$ of the height

- Decisive position and number → 2 stiffening belts



The number of possible combinations complicates design

Summary of structural steel systems by number of storeys



Source: Prof. Z. A. Siddiqi, UET